

# Static calculation of a slab structure using the method of finite elements

Elements:

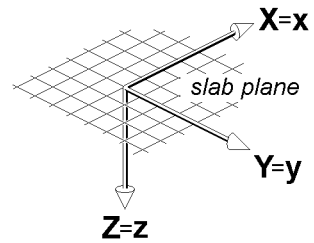
Degrees of freedom of deformations:

Coordinate systems:

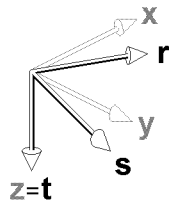
Quadrangular and triangular DKT-elements based on the slab theory of Kirchhoff in conjunction with bar elements of girder grids

Displacement in z-direction, rotation about the x- and y-axis

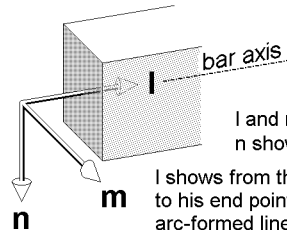
- X-Y-Z** global 3D coordinate system
- x-y-z** coordinate system of the plane
- r-s-t** individual coordinate system of nodes
- l-m-n** coordinate system of bars
- e-f-g** coordinate system of supported lines



All coordinate systems are right handed orthogonal



The r-s-t-system results from a user-defined rotation of the X-Y-Z-system around the z-axis.

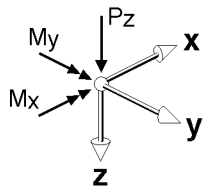


l and m lay in the slab-plane. n shows in z-direction.

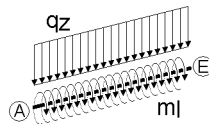
l shows from the start point of the bar to his end point. In case of circular arc-formed lines l tangentially touches the circular arc.

For all nodes, whose r-s-t-system aren't explicit given, r-s-t = x-y-z is valid.

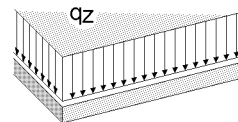
Loads:



Point loads optionally also defined in the r-s-t-system



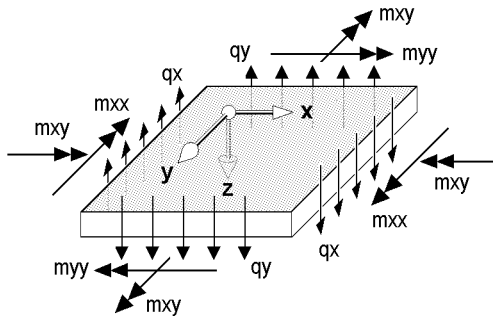
Line loads optionally also linear-variable; pay attention to the line orientation of the torsional moment ml



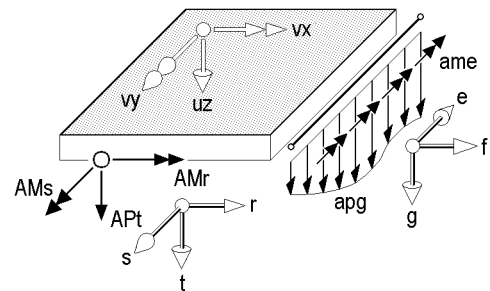
Area loads

Dead loads and area loads act always in z-direction. In case of temperature loads  $\Delta t$  is the difference between the temperature of the upper and the lower extreme fibre.

Results:



- $m_{xx}, m_{yy}$  bending moments [kNm/m]
- $m_{xy}$  torsional moments [kNm/m]
- $q_x, q_y$  shear forces [kN/m]



- $u_z$  displacements [mm]
- $v_x, v_y$  rotations [mm/m]
- $AM_r, AM_s, AP_t$  reactions of supported points [kNm, kN]
- $ame, ap_g$  reactions of supported lines [kNm/m, kN/m]

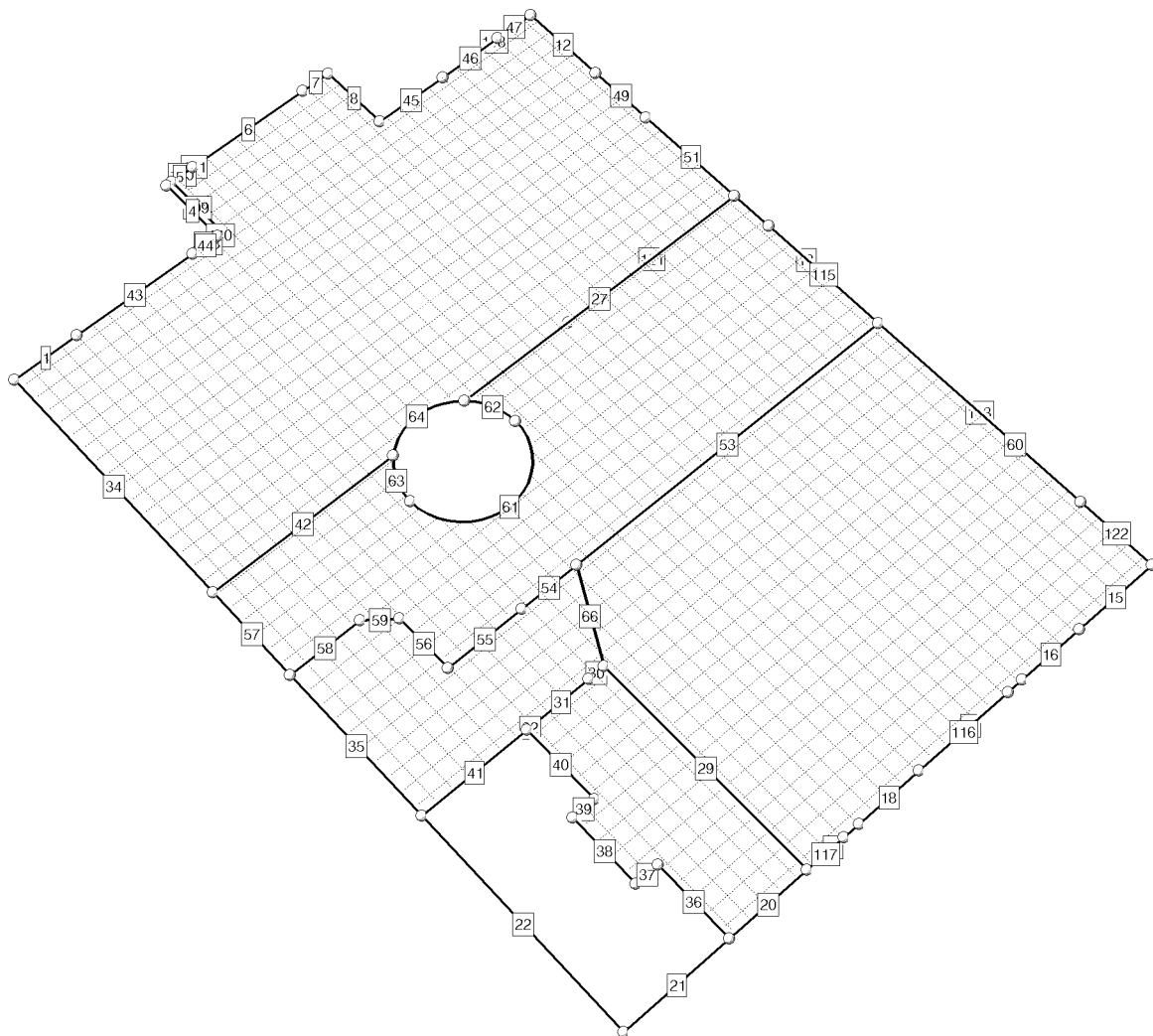
## GLOBAL INFORMATION

### Remarks with respect to calculation

Calculation of the system is linear. Elastic bedding of areas are respected by the method of foundation modulus. The load combinations, which are associated to the demanded verifications, will be described by the defined rules for building extremes as well as the defined load spectra. Remarks with respect to the non-linear behaviour are recorded at this place, but calculation ignores them.

### Overview: Complete system

with line numbers



### Points and coordinates of points in the slab plane

**Type=Rnd:** The point is on the edge of a plane position. **Type=Fix:** The point is part of a plane position and is respected by the mesh generator. **Type=- :** The point is not of relevance to the mesh generator.

point	x m	y m	sheet	type	point	x m	y m	sheet	type
1	2.000	2.000	System	Rnd	8	8.133	0.900	System	Rnd
2	3.000	2.000	System	Rnd	9	8.133	2.000	System	Rnd
3	4.910	2.000	System	Rnd	10	9.263	2.000	System	Rnd
4	5.323	2.000	System	Rnd	11	10.273	2.000	System	Rnd
5	5.323	0.900	System	Rnd	12	10.880	2.000	System	Rnd
6	5.773	0.900	System	Rnd	13	10.880	6.100	System	Rnd
7	7.683	0.900	System	Rnd	14	10.880	8.800	System	Rnd

## Points and coordinates of points in the slab plane

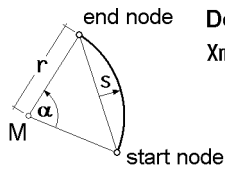
**Type=Rnd:** The point is on the edge of a plane position. **Type=Fix:** The point is part of a plane position and is respected by the mesh generator. **Type= - :** The point is not of relevance to the mesh generator.

point	x	y	sheet	type	point	x	y	sheet	type
-	m	m	-	-	-	m	m	-	-
15	10.880	13.540	System	Rnd	44	5.580	8.800	System	fix
16	9.560	13.540	System	Rnd	45	4.680	8.800	System	fix
17	8.550	13.540	System	Rnd	46	3.480	8.800	System	fix
18	6.770	13.540	System	Rnd	47	3.480	7.900	System	fix
19	5.760	13.540	System	Rnd	48	2.000	7.600	System	Rnd
20	4.905	13.540	System	Rnd	49	3.100	7.600	System	fix
21	3.655	13.540	System	Rnd	50	4.634	6.751	System	Rnd
22	2.005	13.540	System	-	51	6.384	6.751	System	Rnd
27	2.000	6.100	System	Rnd	97	5.010	2.000	LF: 5	-
28	4.905	10.040	System	fix	98	5.410	2.000	LF: 5	-
29	4.655	10.040	System	fix	100	5.410	0.900	LF: 5	-
30	3.755	10.040	System	fix	102	5.810	0.900	LF: 5	-
31	2.005	10.040	System	Rnd	103	7.820	0.900	LF: 5	-
32	3.655	12.340	System	Rnd	105	8.220	0.900	LF: 5	-
33	3.305	12.340	System	Rnd	109	10.880	6.750	LF: 5	-
34	3.305	11.240	System	Rnd	110	10.880	12.350	LF: 5	-
35	3.655	11.240	System	Rnd	111	8.300	13.540	LF: 5	-
36	3.655	10.040	System	Rnd	112	5.510	13.540	LF: 5	-
37	4.900	6.100	System	Rnd	113	10.130	2.000	LF: 5	-
40	6.100	6.100	System	Rnd	118	7.880	6.100	LF: 5	-
41	10.880	3.350	System	Rnd	126	7.820	0.900	LF: 6	-
42	10.880	4.360	System	Rnd	127	8.220	0.900	LF: 6	-

## Straight lines

**Type=Rnd:** The straight line describes the edge of a plane position. **Type=Fix:** The straight line is part of a plane position and is respected by the mesh generator. **Type= - :** The straight line is not of relevance to the mesh generator.

line	start	end	length	sheet	type	line	start	end	length	sheet	type
-	-	-	m	-	-	-	-	-	m	-	-
1	1	2	1.000	System	Rnd	43	2	3	1.910	System	Rnd
4	4	5	1.100	System	Rnd	44	3	4	0.413	System	Rnd
5	5	6	0.450	System	Rnd	45	9	10	1.130	System	Rnd
6	6	7	1.910	System	Rnd	46	10	11	1.010	System	Rnd
7	7	8	0.450	System	Rnd	47	11	12	0.607	System	Rnd
8	8	9	1.100	System	Rnd	49	41	42	1.010	System	Rnd
12	12	41	1.350	System	Rnd	51	42	13	1.740	System	Rnd
13	13	14	2.700	System	Rnd	53	14	44	5.300	System	fix
15	15	16	1.320	System	Rnd	54	44	45	0.900	System	fix
16	16	17	1.010	System	Rnd	55	45	46	1.200	System	fix
17	17	18	1.780	System	Rnd	56	46	47	0.900	System	fix
18	18	19	1.010	System	Rnd	57	27	48	1.500	System	Rnd
19	19	20	0.855	System	Rnd	58	48	49	1.100	System	fix
20	20	21	1.250	System	Rnd	59	49	47	0.484	System	fix
21	21	22	1.650	System	-	60	14	15	4.740	System	Rnd
22	22	31	3.500	System	-	65	28	44	1.412	LF: 3	-
27	13	40	4.780	System	fix	66	28	44	1.412	LF: 4	-
29	20	28	3.500	System	fix	108	97	4	0.313	LF: 5	-
30	28	29	0.250	System	fix	109	98	100	1.100	LF: 5	-
31	29	30	0.900	System	fix	110	100	6	0.363	LF: 5	-
32	30	36	0.100	System	fix	115	109	14	2.050	LF: 5	-
34	27	1	4.100	System	Rnd	116	111	18	1.530	LF: 5	-
35	31	48	2.440	System	Rnd	117	112	20	0.605	LF: 5	-
36	21	32	1.200	System	Rnd	118	113	11	0.143	LF: 5	-
37	32	33	0.350	System	Rnd	121	13	118	3.000	LF: 5	-
38	33	34	1.100	System	Rnd	122	110	15	1.190	LF: 5	-
39	34	35	0.350	System	Rnd	140	4	98	0.087	LF: 6	-
40	35	36	1.200	System	Rnd	141	6	102	0.037	LF: 6	-
41	36	31	1.650	System	Rnd	143	14	110	3.550	LF: 6	-
42	27	37	2.900	System	fix						



### Definition of a circular arc

$X_m$   $Y_m$   $Z_m$  coordinates of the centre point M of the arc  
 $l$  arch length  
 $s$  arch rise  
 $r$  arch radius  
 $\alpha$  interior angle

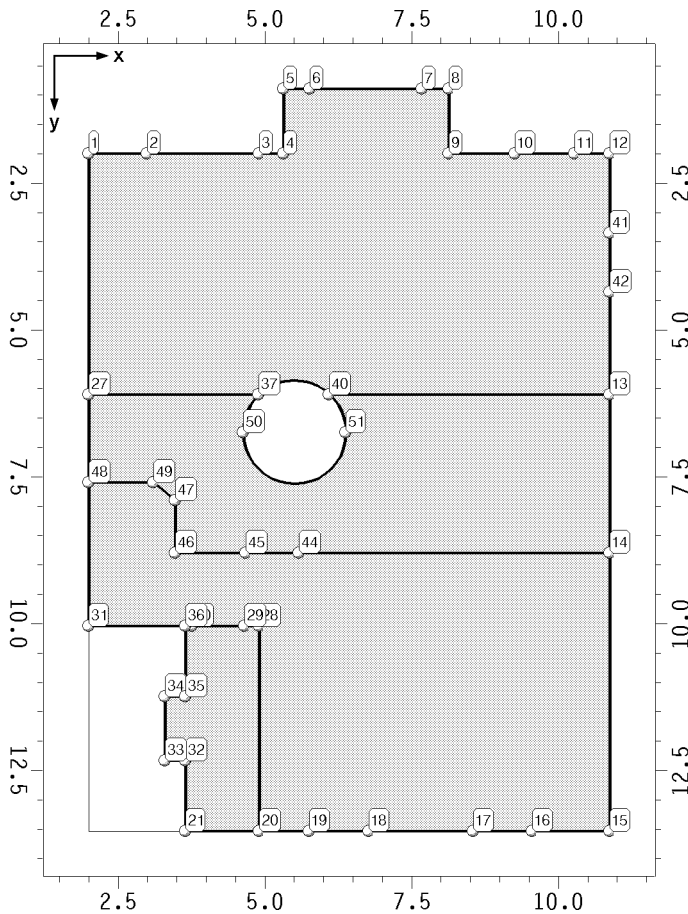
## Circular arcs

**Type=Rnd:** The circular arc describes the edge of plane position . **Type=Fix:** The circular arc is part of a plane position and is respected by the mesh generator. **Type= - :** The circular arc is not of relevance to the mesh generator.

line	start	end	$X_m$	$Y_m$	$Z_m$	$r$	$s$	$l$	$\alpha$	sheet	type
-	-	-	m	m	m	m	m	m	°	-	-
61	50	51	5.509	6.751	0.000	0.875	0.875	2.749	180.000	System	Rnd
62	51	40	5.509	6.745	0.000	0.875	0.075	0.731	47.868	System	Rnd
63	37	50	5.509	6.729	0.000	0.875	0.074	0.724	47.366	System	Rnd
64	40	37	5.500	6.729	0.000	0.869	0.240	1.324	87.306	System	Rnd

## PLANE POSITION 20: POSITION 20 A

### Position 20: Position 20 A in plane: slab plane



## Points in position 20: Position 20 A

x and y are applied to the coordinate system of plane slab plane

**Type=Rnd:** The point is on the edge of the plane position. **Type=Fix:** The point is inside of the plane position and is respected by the mesh generator. **Type= - :** The point is not of relevance to the mesh generator.

point	x	y	type	point	x	y	type	point	x	y	type
-	m	m	-	-	m	m	-	-	m	m	-
1	2.000	2.000	Rnd	16	9.560	13.540	Rnd	36	3.655	10.040	Rnd
2	3.000	2.000	Rnd	17	8.550	13.540	Rnd	37	4.900	6.100	Rnd
3	4.910	2.000	Rnd	18	6.770	13.540	Rnd	40	6.100	6.100	Rnd
4	5.323	2.000	Rnd	19	5.760	13.540	Rnd	41	10.880	3.350	Rnd
5	5.323	0.900	Rnd	20	4.905	13.540	Rnd	42	10.880	4.360	Rnd
6	5.773	0.900	Rnd	21	3.655	13.540	Rnd	44	5.580	8.800	fix
7	7.683	0.900	Rnd	27	2.000	6.100	Rnd	45	4.680	8.800	fix
8	8.133	0.900	Rnd	28	4.905	10.040	fix	46	3.480	8.800	fix
9	8.133	2.000	Rnd	29	4.655	10.040	fix	47	3.480	7.900	fix
10	9.263	2.000	Rnd	30	3.755	10.040	fix	48	2.000	7.600	Rnd
11	10.273	2.000	Rnd	31	2.005	10.040	Rnd	49	3.100	7.600	fix
12	10.880	2.000	Rnd	32	3.655	12.340	Rnd	50	4.634	6.751	Rnd
13	10.880	6.100	Rnd	33	3.305	12.340	Rnd	51	6.384	6.751	Rnd
14	10.880	8.800	Rnd	34	3.305	11.240	Rnd				
15	10.880	13.540	Rnd	35	3.655	11.240	Rnd				

## Definitions of areas

Lines in polygonal order (line-by-line) with indication of orientation (from node - to node)

line	from - to	line	from - to	line	from - to	line	from - to	line	from - to
<b>Edge of plane position 20: Position 20 A</b>									
1	1 2	43	2 3	44	3 4	4	4 5	5	5 6
6	6 7	7	7 8	8	8 9	45	9 10	46	10 11
47	11 12	12	12 41	49	41 42	51	42 13	13	13 14
60	14 15	15	15 16	16	16 17	17	17 18	18	18 19
19	19 20	20	20 21	36	21 32	37	32 33	38	33 34
39	34 35	40	35 36	41	36 31	35	31 48	57	48 27
34	27 1								
<b>Notch</b>									
63	37 50	61	50 51	62	51 40	64	40 37		

## Other lines, which are defined in the position

**Type=Fix:** The line is respected by the mesh generator. **Type= - :** The line is not of relevance to the mesh generator.

line	start	end	type	line	start	end	type	line	start	end	type
-	-	-	-	-	-	-	-	-	-	-	-
27	13	40	fix	32	30	36	fix	55	45	46	fix
29	20	28	fix	42	27	37	fix	56	46	47	fix
30	28	29	fix	53	14	44	fix	58	48	49	fix
31	29	30	fix	54	44	45	fix	59	49	47	fix

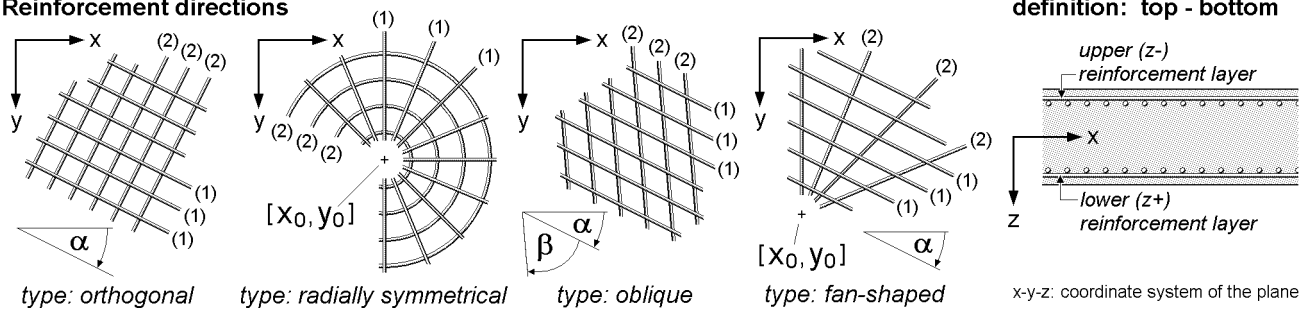
## Calculation values of the position 20: Position 20 A

Material designation: Reinforced concrete C20/25

Geom. characteristic value	Phys. characteristic value	Other characteristic value
Gross area: 100.15 m <sup>2</sup>	E-modulus: 24913.83 MN/m <sup>2</sup>	Length of element edges: 0.35 m
Net area: 97.73 m <sup>2</sup>	Poisson's ratio: 0.20 -	Generation direction: 0.00 °
Circumference: 43.74 m	Temp. coeff.: 1.00 10 <sup>-5</sup> /K	Eccentricity: no
Thickness: 20.00 cm	Bedding: no	

## Explanation of design properties

### Reinforcement directions



## Design properties of plane position 20:

Edge distances	Initial reinforcement	Reinforcement direction	Reinforcement arrangement
(1)top = 3.5 cm	(1)top = 0.00 cm <sup>2</sup> /m	Type: orthogonal with $\alpha = 0.00^\circ$	Tensile reinforcement
(2)top = 4.5 cm	(2)top = 0.00 cm <sup>2</sup> /m		Transformation due to
(1)bottom= 3.5 cm	(1)bottom= 0.00 cm <sup>2</sup> /m		Baumann
(2)bottom= 4.5 cm	(2)bottom= 0.00 cm <sup>2</sup> /m		

## Material properties of position 20:

Material properties for verifications according to DIN 1045-1: concrete C20/25 reinforcement BSt 500  
 Concr.:  $\rho_c = 2200 \text{ kg/m}^3$   $f_{ck} = 20.0 \text{ MN/m}^2$   $\alpha = 0.850$   $\epsilon_{c2} = -2.0\%$   $\epsilon_{c2u} = -3.5\%$   $n_c = 2.00$   
 $E_{cm} = 24913.8 \text{ MN/m}^2$   $f_{ctm} = 2.21 \text{ MN/m}^2$

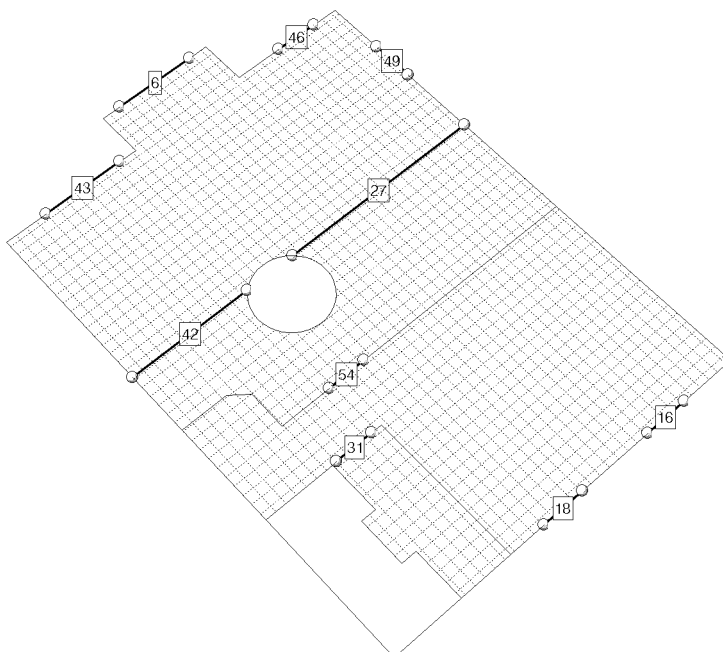
Reinf.:  $f_{yk} = 500.0 \text{ MN/m}^2$   $f_{tk} = 525.0 \text{ MN/m}^2$   $\epsilon_{su} = 25.0\%$   $E_s = 200000.0 \text{ MN/m}^2$

Maximum (rated) reinforcement ratio:  $\max \mu = 8.0\%$

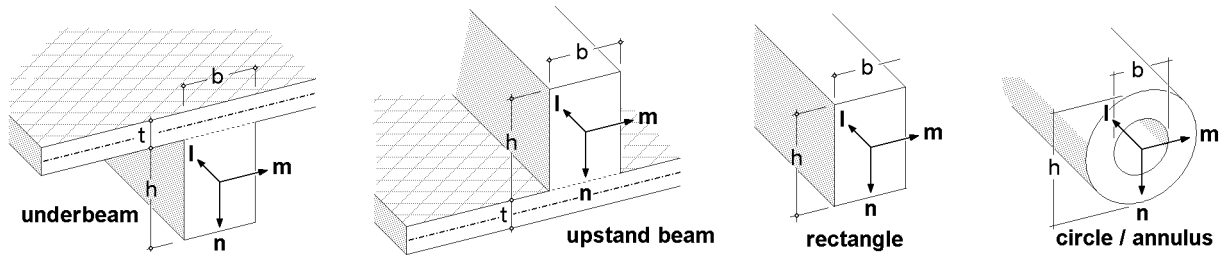
## BARS

### Lines with bar attributes

with line numbers



## Explanations of the bar types



## Description of bars

At haunched beams, subscript A refers to the cross-section of the start node and subscript E to cross-section of the end node.

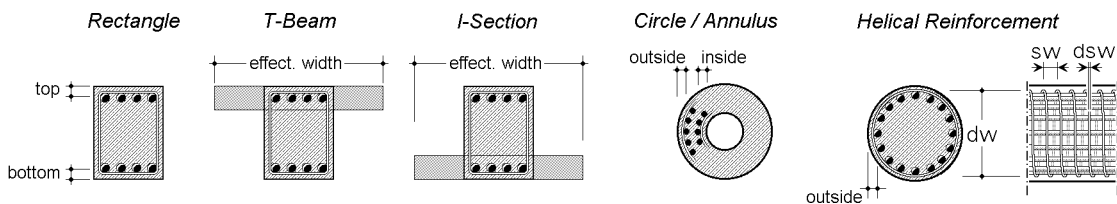
line	start	end	bar type	h cm	b cm	t cm
6	6	7	rectangle	20.0	24.0	---
16	16	17	rectangle	20.0	24.0	---
18	18	19	rectangle	20.0	24.0	---
27	13	40	rectangle	20.0	30.0	---
31	29	30	rectangle	20.0	24.0	---
42	27	37	rectangle	20.0	30.0	---
43	2	3	rectangle	20.0	24.0	---
46	10	11	rectangle	20.0	24.0	---
49	41	42	rectangle	20.0	24.0	---
54	44	45	rectangle	20.0	24.0	---

## Calculation values of bars

At haunched beams, subscript A refers to the cross-section of the start node and subscript E to cross-section of the end node.

line	E-modulus MN/m <sup>2</sup>	$\mu$	$\alpha t$ 10 <sup>-5</sup> /K	I <sub>1</sub> cm <sup>4</sup>	I <sub>m</sub> cm <sup>4</sup>	line	E-modulus MN/m <sup>2</sup>	$\mu$	$\alpha t$ 10 <sup>-5</sup> /K	I <sub>1</sub> cm <sup>4</sup>	I <sub>m</sub> cm <sup>4</sup>
6	24914	0.200	1.000	0	16000	42	24914	0.200	1.000	0	20000
16	24914	0.200	1.000	0	16000	43	24914	0.200	1.000	0	16000
18	24914	0.200	1.000	0	16000	46	24914	0.200	1.000	0	16000
27	24914	0.200	1.000	0	20000	49	24914	0.200	1.000	0	16000
31	24914	0.200	1.000	0	16000	54	24914	0.200	1.000	0	16000

## Explanation of design properties



## Design properties of bars

**Explanations:** column (S) = symmetric condition of reinforcement arrangement: Z = tensile reinforcement, S = symmetric (top = bottom)  
The effective width is only relevant at under-/upstand beams. max  $\rho$  = maximum (rated) reinforcement ratio

bar	edge distances		initial reinf.		S	eff. width		max $\rho$
	top cm	bottom cm	top cm <sup>2</sup>	bottom cm <sup>2</sup>		start cm	end cm	
6	3.5	3.5	0.00	0.00	Z	--	--	8.0
16	3.5	3.5	0.00	0.00	Z	--	--	8.0
18	3.5	3.5	0.00	0.00	Z	--	--	8.0
27	5.0	5.0	0.00	0.00	Z	--	--	8.0
31	3.5	3.5	0.00	0.00	Z	--	--	8.0
42	5.0	5.0	0.00	0.00	Z	--	--	8.0
43	5.0	5.0	0.00	0.00	Z	--	--	8.0

## Design properties of bars

**Explanations:** column (S) = symmetric condition of reinforcement arrangement: Z = tensile reinforcement, S = symmetric (top = bottom)  
The effective width is only relevant at under-/upstand beams. max  $\rho$  = maximum (rated) reinforcement ratio

bar	edge distances		initial reinf.		S	eff. width		max $\rho$
	top	bottom	top	bottom		start	end	
	cm	cm	cm <sup>2</sup>	cm <sup>2</sup>		cm	cm	
46	5.0	5.0	0.00	0.00	Z	--	--	8.0
49	5.0	5.0	0.00	0.00	Z	--	--	8.0
54	5.0	5.0	0.00	0.00	Z	--	--	8.0

## Material properties of bars for verifications according to DIN 1045-1

**Explanations:**  $\rho_c$ : maximum density of concrete; BSt<sub>l</sub>: steel quality of longitudinal reinforcement

Material properties of concrete:  $f_{ck}$ : compressive strength of cylinder;  $\alpha_c$ : reduction factor (eqn. 67);  $\epsilon_{c2}$ ,  $\epsilon_{c2u}$ : strains;

$n_c$ : exponent to describe the stress-strain-diagram (eqn. 65);  $E_{cm}$ : mean elastic modulus (secant modulus)

$f_{ctm}$ : mean value of centric tensile strength; calculation of deformations: final creep coefficient  $\varphi_{\infty,10}$ ; final shrinkage strain  $\epsilon_{cs}$ ;

Exposure classes for reinforcement corrosion XC, concrete attack XF, concrete corrosion (moisture class AKR) W

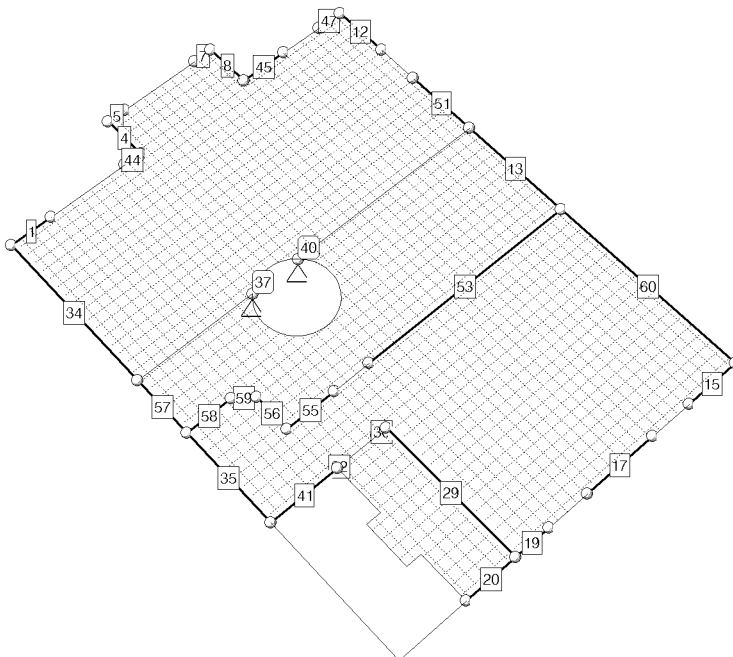
Material properties of reinforcement:  $f_{yk}$ : yield strength;  $f_{tk}$ : tensile strength;  $\epsilon_{su}$ : elongation at failure;  $E_s$ : elastic modulus

bar	concrete	$\rho_c$	BSt <sub>l</sub>	$f_{ck}$	$\alpha_c$	$\epsilon_{c2}$	$\epsilon_{c2u}$	$n_c$	$E_{cm}$	$f_{ctm}$	$\varphi_{\infty,10}$	$\epsilon_{cs}$	$f_{yk}$	$f_{tk}$	$\epsilon_{su}$	$E_s$	XC	XF	W
		kg/m <sup>3</sup>		MN/m <sup>2</sup>		%	%		MN/m <sup>2</sup>	MN/m <sup>2</sup>	---	---	MN/m <sup>2</sup>	MN/m <sup>2</sup>	%	MN/m <sup>2</sup>			
6	C25/30	2200	500	25.0	0.850	-2.0	-3.5	2.00	26662.6	2.56	---	---	500.0	525.0	25.0	200000.0	---	---	---
16	C25/30	2200	500	25.0	0.850	-2.0	-3.5	2.00	26662.6	2.56	---	---	500.0	525.0	25.0	200000.0	---	---	---
18	C25/30	2200	500	25.0	0.850	-2.0	-3.5	2.00	26662.6	2.56	---	---	500.0	525.0	25.0	200000.0	---	---	---
27	C20/25	2200	500	20.0	0.850	-2.0	-3.5	2.00	24913.8	2.21	---	---	500.0	525.0	25.0	200000.0	---	---	---
31	C25/30	2200	500	25.0	0.850	-2.0	-3.5	2.00	26662.6	2.56	---	---	500.0	525.0	25.0	200000.0	---	---	---
42	C20/25	2200	500	20.0	0.850	-2.0	-3.5	2.00	24913.8	2.21	---	---	500.0	525.0	25.0	200000.0	---	---	---
43	C20/25	2200	500	20.0	0.850	-2.0	-3.5	2.00	24913.8	2.21	---	---	500.0	525.0	25.0	200000.0	---	---	---
46	C20/25	2200	500	20.0	0.850	-2.0	-3.5	2.00	24913.8	2.21	---	---	500.0	525.0	25.0	200000.0	---	---	---
49	C20/25	2200	500	20.0	0.850	-2.0	-3.5	2.00	24913.8	2.21	---	---	500.0	525.0	25.0	200000.0	---	---	---
54	C20/25	2200	500	20.0	0.850	-2.0	-3.5	2.00	24913.8	2.21	---	---	500.0	525.0	25.0	200000.0	---	---	---

## BEARING INFORMATION

### Supported lines and points

with numbers of lines and points





## Supported lines

Cug: spring constants against a displacement in z-direction. Cve: spring constants against a rotation about the longitudinal axis. Cvf: spring constants against a rotation about the transversal axis. In case of non-linear calculation the denoted disability of displacement takes effect only by: (1) positive displacements, (2) negative displacements, (3) always.

line	Cug	Cve	Cvf	line	Cug	Cve	Cvf
-	MN/m <sup>2</sup>	MNm/m	MNm/m	-	MN/m <sup>2</sup>	MNm/m	MNm/m
1	<rigid>(1)	--	--	34	<rigid>(1)	--	--
4	<rigid>(1)	--	--	35	<rigid>(1)	--	--
5	<rigid>(1)	--	--	41	<rigid>(1)	--	--
7	<rigid>(1)	--	--	44	<rigid>(1)	--	--
8	<rigid>(1)	--	--	45	<rigid>(1)	--	--
12	<rigid>(1)	--	--	47	<rigid>(1)	--	--
13	<rigid>(1)	--	--	51	<rigid>(1)	--	--
15	<rigid>(1)	--	--	53	2000.000(1)	--	--
17	<rigid>(1)	--	--	55	2000.000(1)	--	--
19	<rigid>(1)	--	--	56	2000.000(1)	--	--
20	<rigid>(1)	--	--	57	<rigid>(1)	--	--
29	<rigid>(1)	--	--	58	2000.000(1)	--	--
30	<rigid>(1)	--	--	59	2000.000(1)	--	--
32	<rigid>(1)	--	--	60	<rigid>(1)	--	--

## Supported points

Cut: spring constants against a displacement in z-direction. Cvr: spring constants against a rotation about the r-axis. Cvs: spring constants against a rotation about the s-axis. In case of non-linear calculation the denoted disability of displacement takes effect only by: (1) positive displacements, (2) negative displacements, (3) always.

point	Cut	Cvr	Cvs
-	MN/m	MNm	MNm
37	<rigid>(1)	--	--
40	<rigid>(1)	--	--

## LOADING STRUCTURE

### Description of loading structure

On the left-hand side, the relationship between the actions effects, load case file and load cases are shown in a tree structure. The right-hand side shows the characteristics of the superposition to the associated objects on the left-hand. In terms of the superposition, a load case file is equivalent to an extreme rule of the defined objects therein and can be additive or alternatively superpositioned.

Used symbols:  Action effect  Load case file  Load case

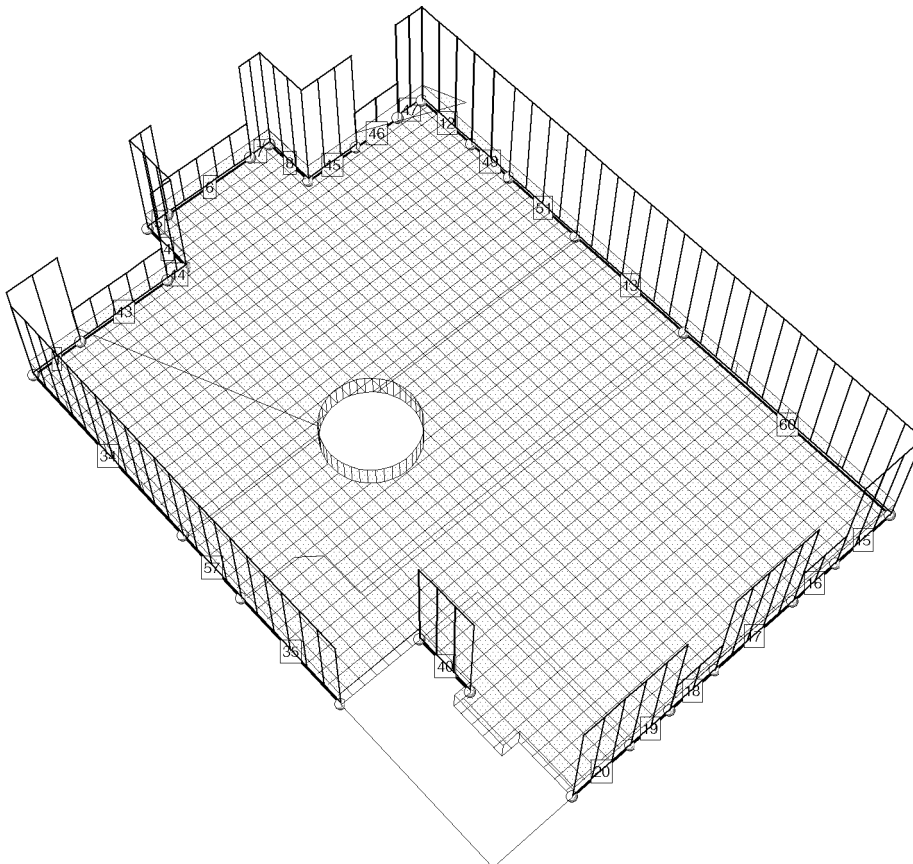
## Description of loading structure

On the left-hand side, the relationship between the actions effects, load case file and load cases are shown in a tree structure. The right-hand side shows the characteristics of the superposition to the associated objects on the left-hand. In terms of the superposition, a load case file is equivalent to an extreme rule of the defined objects therein and can be additive or alternatively superpositioned.

<ul style="list-style-type: none"> <li>1: permanent loads           <ul style="list-style-type: none"> <li>1: dead load (1)</li> <li>5: Load spectrum 1</li> </ul> </li> <li>2: live loads (1)           <ul style="list-style-type: none"> <li>2: live loads (1/1)</li> <li>3: live loads (1/2)</li> <li>4: live loads (1/3)</li> <li>6: Load spectrum 2</li> </ul> </li> </ul>	<p><b>permanent loads</b></p> <p>additive</p> <p>additive</p> <p><b>transient live loads of housing, office rooms</b></p> <p>additive</p> <p>additive</p> <p>additive</p> <p>additive</p>
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## LOAD PICTURES IN LOAD CASE 1: DEAD LOAD (1)

### Loaded objects in load case 1



### Denoted loaded objects

type	number	designation
position	20	Position 20 A

## Density of expelled areas in load case 1

area type	no. designation	$\gamma$ kN/m <sup>3</sup>
-	-	-
position	20 Position 20 A	25.000

## Area loads in load case 1

Linear transient area loads are defined by inputting load ordinates at 3 different points.

area type	no. designation	at point	$q_z$ kN/m <sup>2</sup>
-	-	-	-
position	20 Position 20 A	const.	1.500

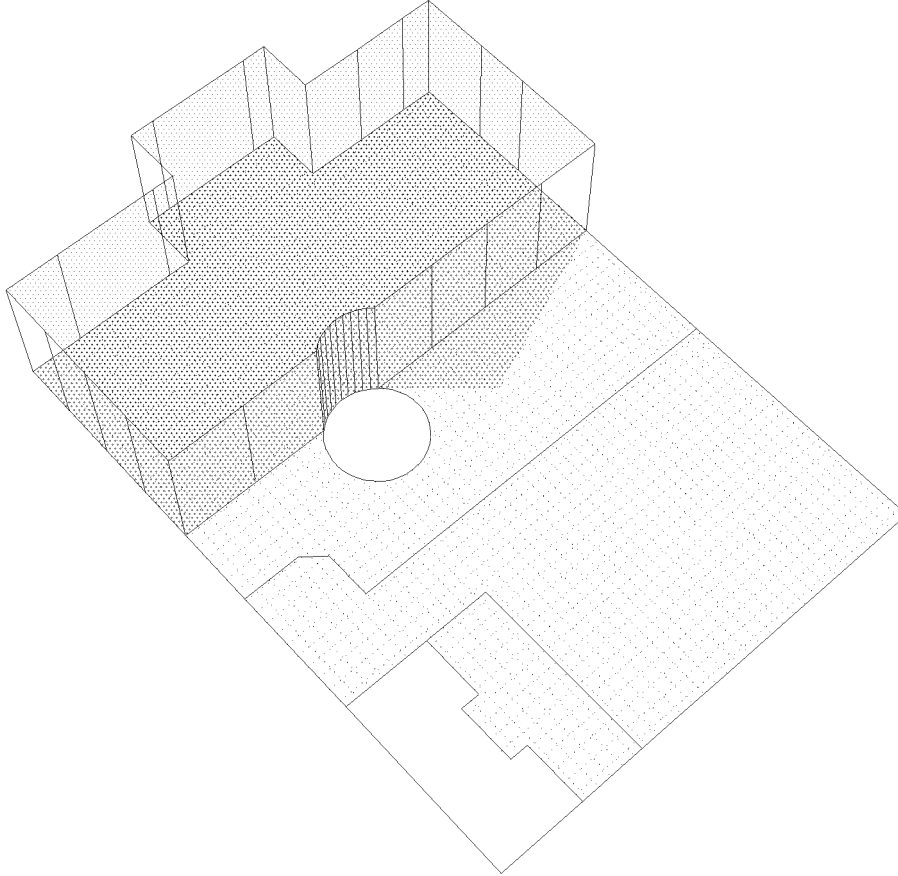
## Line loads in load case 1

At variable line loads, subscript A refers to the coordinates of the start node and subscript E to coordinates of the end node.

line	start	end	$q_z$ kN/m	$m_1$ kNm/m	line	start	end	$q_z$ kN/m	$m_1$ kNm/m
-	-	-	-	-	-	-	-	-	-
40	35	36	8.100	0.000	5	5	6	8.200	0.000
35	31	48	6.860	0.000	6	6	7	3.000	0.000
57	27	48	6.860	0.000	7	7	8	8.200	0.000
34	27	1	6.860	0.000	8	8	9	8.200	0.000
60	14	15	8.200	0.000	45	9	10	8.200	0.000
13	13	14	8.200	0.000	46	10	11	3.000	0.000
51	42	13	8.200	0.000	47	11	12	8.200	0.000
49	41	42	8.200	0.000	20	20	21	8.200	0.000
12	12	41	8.200	0.000	19	19	20	8.200	0.000
1	1	2	8.200	0.000	18	18	19	3.000	0.000
43	2	3	3.000	0.000	17	17	18	8.200	0.000
44	3	4	8.200	0.000	16	16	17	3.000	0.000
4	4	5	8.200	0.000	15	15	16	8.200	0.000

## LOAD PICTURES IN LOAD CASE 2: LIVE LOADS (1/1)

### Loaded objects in load case 2



### Denoted loaded objects

type	number	designation
Load area	1	neue Load area

### Edge descriptions of load areas

Lines in polygonal order (line-by-line) with indication of orientation (from node - to node)

line	from - to	line	from - to	line	from - to	line	from - to	line	from - to
Load area 1: neue Load area in plane slab plane									
42	27 37	64	37 40	27	40 13	51	13 42	49	42 41
12	41 12	47	12 11	46	11 10	45	10 9	8	9 8
7	8 7	6	7 6	5	6 5	4	5 4	44	4 3
43	3 2	1	2 1	34	1 27				

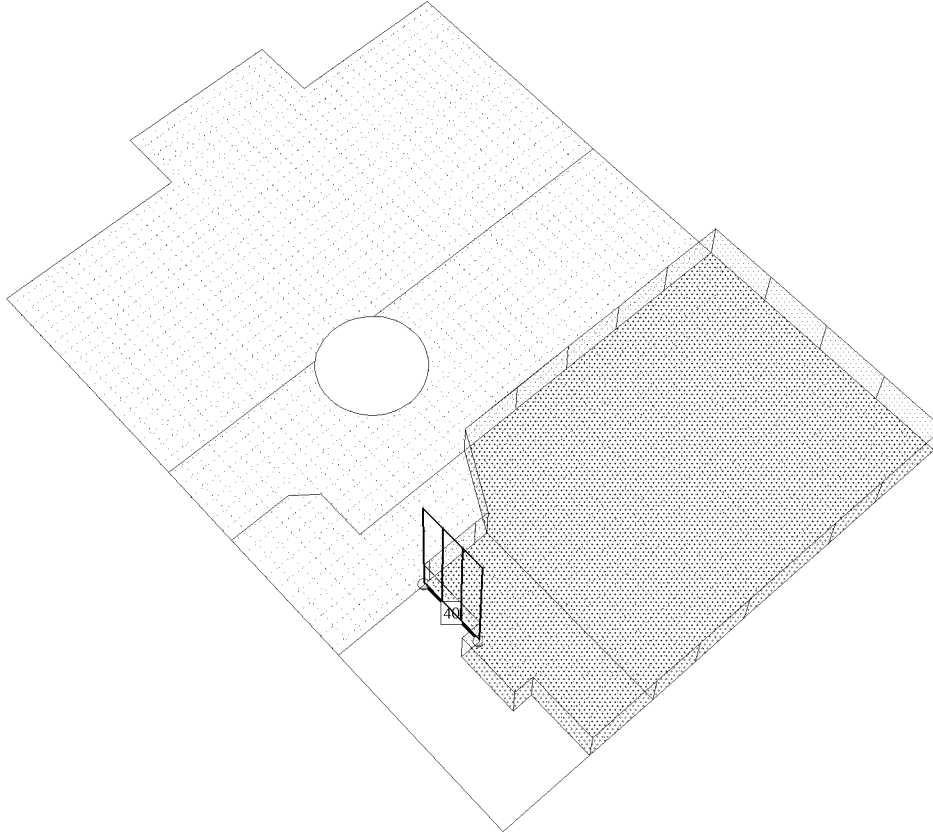
### Area loads in load case 2

Linear transient area loads are defined by inputting load ordinates at 3 different points.

area type	no.	designation	at point	qz
-	-	-	-	kN/m <sup>2</sup>
Load area	1	neue Load area	const.	2.700

## LOAD PICTURES IN LOAD CASE 3: LIVE LOADS (1/2)

### Loaded objects in load case 3



### Denoted loaded objects

type	number	designation
Load area	2	neue Load area

### Edge descriptions of load areas

Lines in polygonal order (line-by-line) with indication of orientation (from node - to node)

line	from - to	line	from - to	line	from - to	line	from - to	line	from - to
Load area 2: neue Load area in plane slab plane									
65	28 44	53	44 14	60	14 15	15	15 16	16	16 17
17	17 18	18	18 19	19	19 20	20	20 21	36	21 32
37	32 33	38	33 34	39	34 35	40	35 36	32	36 30
31	30 29	30	29 28						

### Area loads in load case 3

Linear transient area loads are defined by inputting load ordinates at 3 different points.

area type	no.	designation	at point	qz
-	-	-	-	kN/m2
Load area	2	neue Load area	const.	2.700

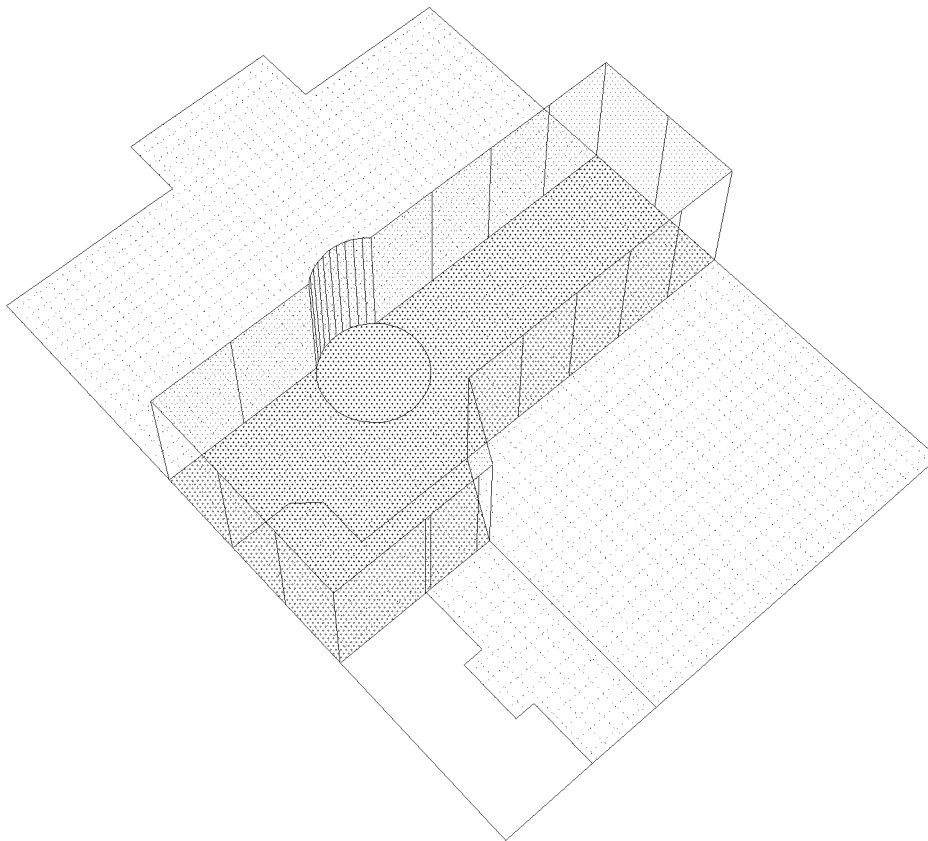
### Line loads in load case 3

At variable line loads, subscript A refers to the coordinates of the start node and subscript E to coordinates of the end node.

line	start	end	qz kN/m	m1 kNm/m
40	35	36	9.400	0.000

### LOAD PICTURES IN LOAD CASE 4: LIVE LOADS (1/3)

#### Loaded objects in load case 4



#### Denoted loaded objects

type	number	designation
Load area	3	neue Load area

#### Edge descriptions of load areas

Lines in polygonal order (line-by-line) with indication of orientation (from node - to node)

line	from - to	line	from - to	line	from - to	line	from - to	line	from - to
Load area 3: neue		Load area in plane		slab plane					
66	28 44	53	44 14	13	14 13	27	13 40	64	40 37
42	37 27	57	27 48	35	48 31	41	31 36	32	36 30
31	30 29	30	29 28						

## Area loads in load case 4

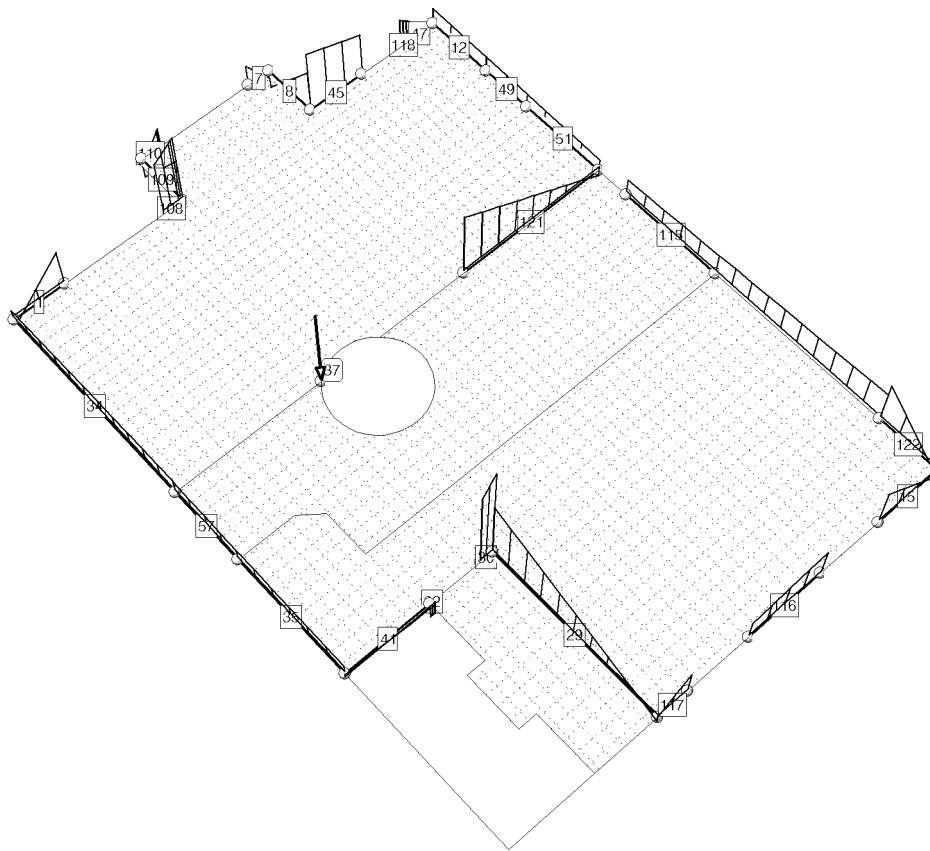
Linear transient area loads are defined by inputting load ordinates at 3 different points.

area type	no. designation	at point	qz kN/m <sup>2</sup>
Load area	3 neue Load area	const.	2.700

## LOAD PICTURES IN LOAD CASE 5: LOAD SPECTRUM 1

### Loaded objects in load case 5

The pictures of these load cases were imported from the support reactions of the building component "Position 30 A".



### Line loads in load case 5

At variable line loads, subscript A refers to the coordinates of the start node and subscript E to coordinates of the end node.

line	start	end	qz kN/m	m1 kNm/m	line	start	end	qz kN/m	m1 kNm/m
1A	1	-	-22.425	0.000	115E	-	14	35.240	0.000
1E	-	2	65.385	0.000	15A	15	-	-27.538	0.000
108A	97	-	102.298	0.000	15E	-	16	66.617	0.000
108E	-	4	124.875	0.000	116A	111	-	50.042	0.000
109A	98	-	79.980	0.000	116E	-	18	31.585	0.000
109E	-	100	-49.574	0.000	117A	112	-	40.240	0.000
110A	100	-	-24.717	0.000	117E	-	20	-7.717	0.000
110E	-	6	25.856	0.000	118A	113	-	49.483	0.000
12A	12	-	24.342	0.000	118E	-	11	37.718	0.000
12E	-	41	21.501	0.000	121A	13	-	-19.569	0.000
115A	109	-	25.123	0.000	121E	-	118	119.815	0.000

## Line loads in load case 5

At variable line loads, subscript A refers to the coordinates of the start node and subscript E to coordinates of the end node.

line	start	end	qz	m1	line	start	end	qz	m1
-	-	-	kN/m	kNm/m	-	-	-	kN/m	kNm/m
29 <sub>A</sub>	20		-18.540	0.000	8 <sub>E</sub>		9	73.331	0.000
29 <sub>E</sub>		28	111.910	0.000	45 <sub>A</sub>	9		112.132	0.000
30 <sub>A</sub>	28		197.574	0.000	45 <sub>E</sub>		10	75.350	0.000
30 <sub>E</sub>		29	159.844	0.000	140 <sub>A</sub>	4		124.875	0.000
32 <sub>A</sub>	30		-31.458	0.000	140 <sub>E</sub>		98	131.105	0.000
32 <sub>E</sub>		36	-30.196	0.000	141 <sub>A</sub>	6		25.856	0.000
122 <sub>A</sub>	110		75.272	0.000	141 <sub>E</sub>		102	30.936	0.000
122 <sub>E</sub>		15	-21.385	0.000	47 <sub>A</sub>	11		37.718	0.000
34 <sub>A</sub>	27		22.482	0.000	47 <sub>E</sub>		12	-12.011	0.000
34 <sub>E</sub>		1	13.516	0.000	143 <sub>A</sub>	14		35.240	0.000
35 <sub>A</sub>	31		14.286	0.000	143 <sub>E</sub>		110	52.759	0.000
35 <sub>E</sub>		48	17.011	0.000	49 <sub>A</sub>	41		21.501	0.000
41 <sub>A</sub>	36		-6.615	0.000	49 <sub>E</sub>		42	19.375	0.000
41 <sub>E</sub>		31	5.674	0.000	51 <sub>A</sub>	42		19.375	0.000
7 <sub>A</sub>	7		32.492	0.000	51 <sub>E</sub>		13	15.713	0.000
7 <sub>E</sub>		8	-23.382	0.000	57 <sub>A</sub>	27		18.686	0.000
8 <sub>A</sub>	8		-43.881	0.000	57 <sub>E</sub>		48	17.011	0.000

## Point loads in load case 5

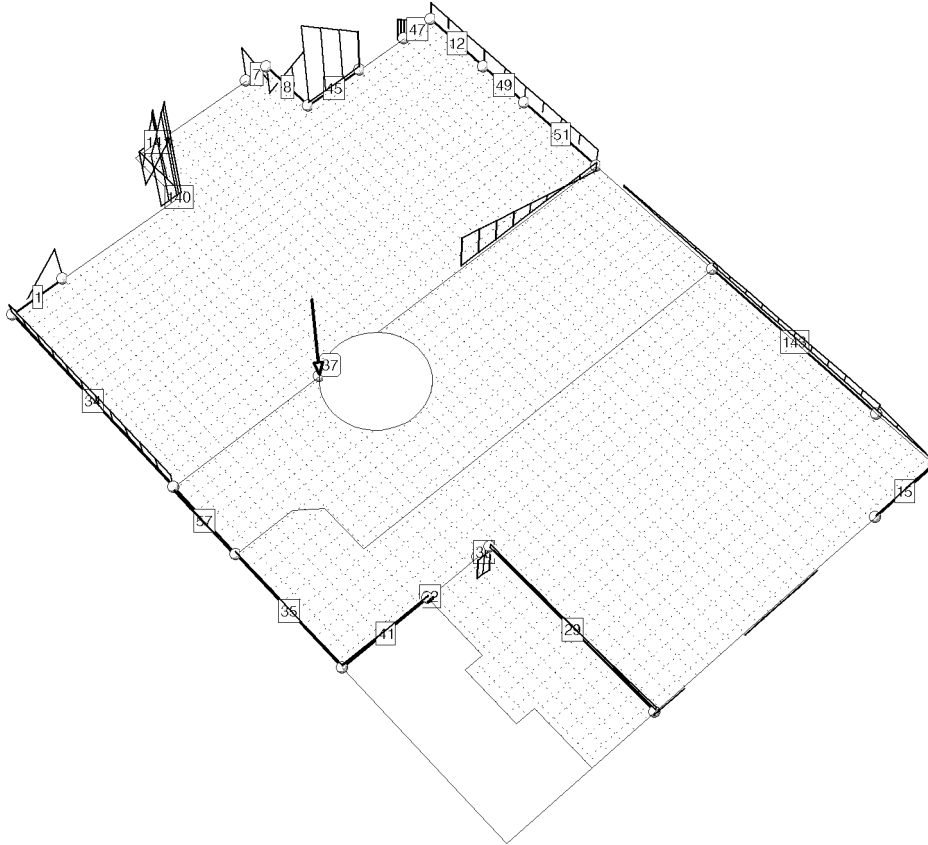
point	syst.	P <sub>z</sub> (P <sub>t</sub> )	M <sub>x</sub> (M <sub>r</sub> )	M <sub>y</sub> (M <sub>s</sub> )
-	-	kN	kNm	kNm
37	r-s-t	153.425	0.000	0.000



## LOAD PICTURES IN LOAD CASE 6: LOAD SPECTRUM 2

### Loaded objects in load case 6

The pictures of these load cases were imported from the support reactions of the building component "Position 30 A".



### Line loads in load case 6

At variable line loads, subscript A refers to the coordinates of the start node and subscript E to coordinates of the end node.

line	start	end	qz	m1	line	start	end	qz	m1
-	-	-	kN/m	kNm/m	-	-	-	kN/m	kNm/m
1 <sub>A</sub>	1		-13.334	0.000	29 <sub>A</sub>	20		4.006	0.000
1 <sub>E</sub>		2	34.317	0.000	29 <sub>E</sub>		28	-4.665	0.000
108 <sub>A</sub>	97		60.954	0.000	30 <sub>A</sub>	28		-30.953	0.000
108 <sub>E</sub>		4	96.472	0.000	30 <sub>E</sub>		29	-28.173	0.000
109 <sub>A</sub>	98		66.737	0.000	32 <sub>A</sub>	30		-2.729	0.000
109 <sub>E</sub>		100	-41.795	0.000	32 <sub>E</sub>		36	-2.422	0.000
110 <sub>A</sub>	100		-21.730	0.000	122 <sub>A</sub>	110		6.981	0.000
110 <sub>E</sub>		6	28.448	0.000	122 <sub>E</sub>		15	-1.526	0.000
12 <sub>A</sub>	12		15.473	0.000	34 <sub>A</sub>	27		13.534	0.000
12 <sub>E</sub>		41	15.441	0.000	34 <sub>E</sub>		1	9.172	0.000
115 <sub>A</sub>	109		1.419	0.000	35 <sub>A</sub>	31		0.172	0.000
115 <sub>E</sub>		14	4.190	0.000	35 <sub>E</sub>		48	-1.499	0.000
15 <sub>A</sub>	15		1.104	0.000	41 <sub>A</sub>	36		-1.071	0.000
15 <sub>E</sub>		16	-2.437	0.000	41 <sub>E</sub>		31	1.927	0.000
116 <sub>A</sub>	111		-0.763	0.000	7 <sub>A</sub>	7		34.937	0.000
116 <sub>E</sub>		18	-0.153	0.000	7 <sub>E</sub>		8	-20.347	0.000
117 <sub>A</sub>	112		-1.463	0.000	8 <sub>A</sub>	8		-36.108	0.000
117 <sub>E</sub>		20	0.390	0.000	8 <sub>E</sub>		9	60.038	0.000
118 <sub>A</sub>	113		22.811	0.000	45 <sub>A</sub>	9		87.347	0.000
118 <sub>E</sub>		11	17.304	0.000	45 <sub>E</sub>		10	40.430	0.000
121 <sub>A</sub>	13		-10.175	0.000	140 <sub>A</sub>	4		96.472	0.000
121 <sub>E</sub>		118	34.363	0.000	140 <sub>E</sub>		98	106.274	0.000

## Line loads in load case 6

At variable line loads, subscript A refers to the coordinates of the start node and subscript E to coordinates of the end node.

line	start	end	qz	m1	line	start	end	qz	m1
-	-	-	kN/m	kNm/m	-	-	-	kN/m	kNm/m
141 <sub>A</sub>	6		28.448	0.000	49 <sub>A</sub>	41		15.441	0.000
141 <sub>E</sub>		102	33.488	0.000	49 <sub>E</sub>		42	15.417	0.000
47 <sub>A</sub>	11		17.304	0.000	51 <sub>A</sub>	42		15.417	0.000
47 <sub>E</sub>		12	-5.972	0.000	51 <sub>E</sub>		13	15.375	0.000
143 <sub>A</sub>	14		4.190	0.000	57 <sub>A</sub>	27		-2.527	0.000
143 <sub>E</sub>		110	8.989	0.000	57 <sub>E</sub>		48	-1.499	0.000

## Point loads in load case 6

point	syst.	Pz (Pt)	Mx (Mr)	My (Ms)
-	-	kN	kNm	kNm
37	r-s-t	97.216	0.000	0.000

## DESCRIPTION OF DEMANDED VERIFICATIONS

The following means:

- $\Psi_{dom}$  in rule of superposition DIN 1055-100: Combination coefficient of a leading traffic load action  
in rule of superposition DIN 18800: Combination coefficient of a minor combination
- $\Psi_{sub}$  in rule of superposition DIN 1055-100: Combination coefficient of a non-leading traffic load action  
in rule of superposition DIN 18800: Combination coefficient of a main combination
- $\gamma_{sup}$  Partial safety factor for unfavourable load positions
- $\gamma_{inf}$  Partial safety factor for favourable load positions

Rules of superposition FB101 and Eurocode are comparable with DIN 1055-100  
In non-linear analysis, rules of extremization will not be considered

## Verification 1: DIN 1045-1 Design calculation

DIN 1045-1 Design calculation: Design resistance according to DIN 1045-1 (10.2, 10.3, 10.4)

Design options to verification 1:

- Bending design calculation
- Shear design calculation
  - z from Bending design calculation
  - z = 0.9 d ≤ d - 2 c<sub>nom</sub>
  - z of bending design calc. ≤ d - 2 c<sub>nom</sub>
  - Design calculation in reinforcement directions
  - Design calculation in main shear direction
  - NO limitation of VR<sub>dct</sub>
- with minimum/transverse reinforcement (bending, shear)
- DIN 1045-1 (7.01) incl. corr.2 (6.05)
- DIN 1045-1 (8.08)

## 1: Standard combination

Rules of extremization to verification 1, type: standard, Rule of superposition: DIN1055-100

Safety factor of material:  $\gamma_C = 1.50$ ,  $\gamma_S = 1.15$

action	$\Psi_{dom}$	$\Psi_{sub}$	$\gamma_{sup}$	$\gamma_{inf}$
1	1.00	1.00	1.35	1.00
2	1.00	0.70	1.50	0.00

### Table of design plane positions (Verification 1)

**Explanations:** column (M): minimum reinforcement of slabs; column (Q): transverse reinf. - minimum portions of main reinf.  
 column (S): shear design calc. (with 'no' resp. 'with' minimum reinf. of shear); column (P): if possible, avoid shear reinf. (by increasing longitud. reinf.)  
 BSt<sub>l</sub>, BSt<sub>q</sub>: steel quality of longitudinal, shear reinforcement ('Gitter': synonym of lattice girder  
 with  $f_{yk} = 420 \text{ MN/m}^2$ . Verifications basing on approval documents are NOT kept !);  $c_{v,D}$ : concrete cover of compression reinf.;  
 $\Theta$ : angle of compression strut (0 = minimum);  $\alpha_q$ : angle of shear reinforcement; column (F): joint; column (O): finish of joint  
 Description of material see: 'Material properties of plane position'

pos.	concrete	BSt <sub>l</sub>	(M)	(Q)	(S)	BSt <sub>q</sub>	$c_{v,D}$ cm	$\Theta$ °	(P)	$\alpha_q$ °	(F)	(O)
20	C20/25	500	no	0.20	no	----	---	---	--	---	----	----

### Table of design bars (Verification 1)

**Explanations:** column (M): minimum reinforcement of beams

$b_j$ : breadth of joints (0 = web breadth); column (W): efficiency factor of stirrups in circular cross-sections;  $t_{eff}$ : torsion, effect. wall thickness (0 = according to design)  
 more explanations s. plane positions; description of material see: 'Material properties of bars'

bar	concrete	BSt <sub>l</sub>	(M)	(S)	BSt <sub>q</sub>	$c_{v,D}$ cm	$\Theta$ °	(F)	(O)	$b_j$ cm	(W)	$t_{eff}$ cm
6	C25/30	500	yes	with	500	3.0	0	no	---	--	--	0.0
16	C25/30	500	yes	with	500	3.0	0	no	---	--	--	0.0
18	C25/30	500	yes	with	500	3.0	0	no	---	--	--	0.0
27	C20/25	500	yes	with	500	3.0	0	no	---	--	--	0.0
31	C25/30	500	yes	with	500	3.0	0	no	---	--	--	0.0
42	C20/25	500	yes	with	500	3.0	0	no	---	--	--	0.0
43	C20/25	500	yes	with	500	3.0	0	no	---	--	--	0.0
46	C20/25	500	yes	with	500	3.0	0	no	---	--	--	0.0
49	C20/25	500	yes	with	500	3.0	0	no	---	--	--	0.0
54	C20/25	500	yes	with	500	3.0	0	no	---	--	--	0.0

## Verification 2: Analysis

Analysis: Analysis without verifications

### 1: Standard combination

Rules of extremization to verification 2, type: standard, Rule of superposition: old design code

action	$\gamma_{sup}$	$\gamma_{inf}$
1	1.00	1.00
2	1.00	0.00

## Verification 3: Export of support reactions

Export of support reactions: Export of reactions of supported lines and points

### Load spectra to verification 3

Factorization of load cases. Negative numbers of load cases refer to imperfections

LK	1	2	3	4	5	6
1	1.00	-	-	-	1.00	-
2	-	1.00	1.00	1.00	-	1.00

