

POS. 15: WAGENKNECHT BD.2, BSP.11.10.3

detailed problems acc. to Eurocode 3

EC 3-1-5 (12.10), NA: Deutschland

steel grade

steel grade S 235

cross-section

beam: section parameters (box section):

$h = 360.0 \text{ mm}$, $t_w = 6.0 \text{ mm}$ (2x), $b_f = 360.0 \text{ mm}$, $b_{\bar{u}} = 0.0 \text{ mm}$, $t_f = 6.0 \text{ mm}$

parameters

length of buckling field $a = 800.0 \text{ cm}$

method of effective cross-sectional area

verification in beam field

calculation of buckling factors acc. to EC 3-1-5

effective cross-sectional properties: A_{eff} solely from compression, W_{eff} solely from bending

verification of stability acc. to EC 3-1-1, 6.3

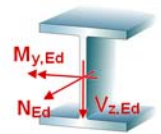
loading

Lk 1: $N_{\text{Ed}} = -950.0 \text{ kN}$

partial safety factors for material

resistance of cross-sections $\gamma_{M0} = 1.00$

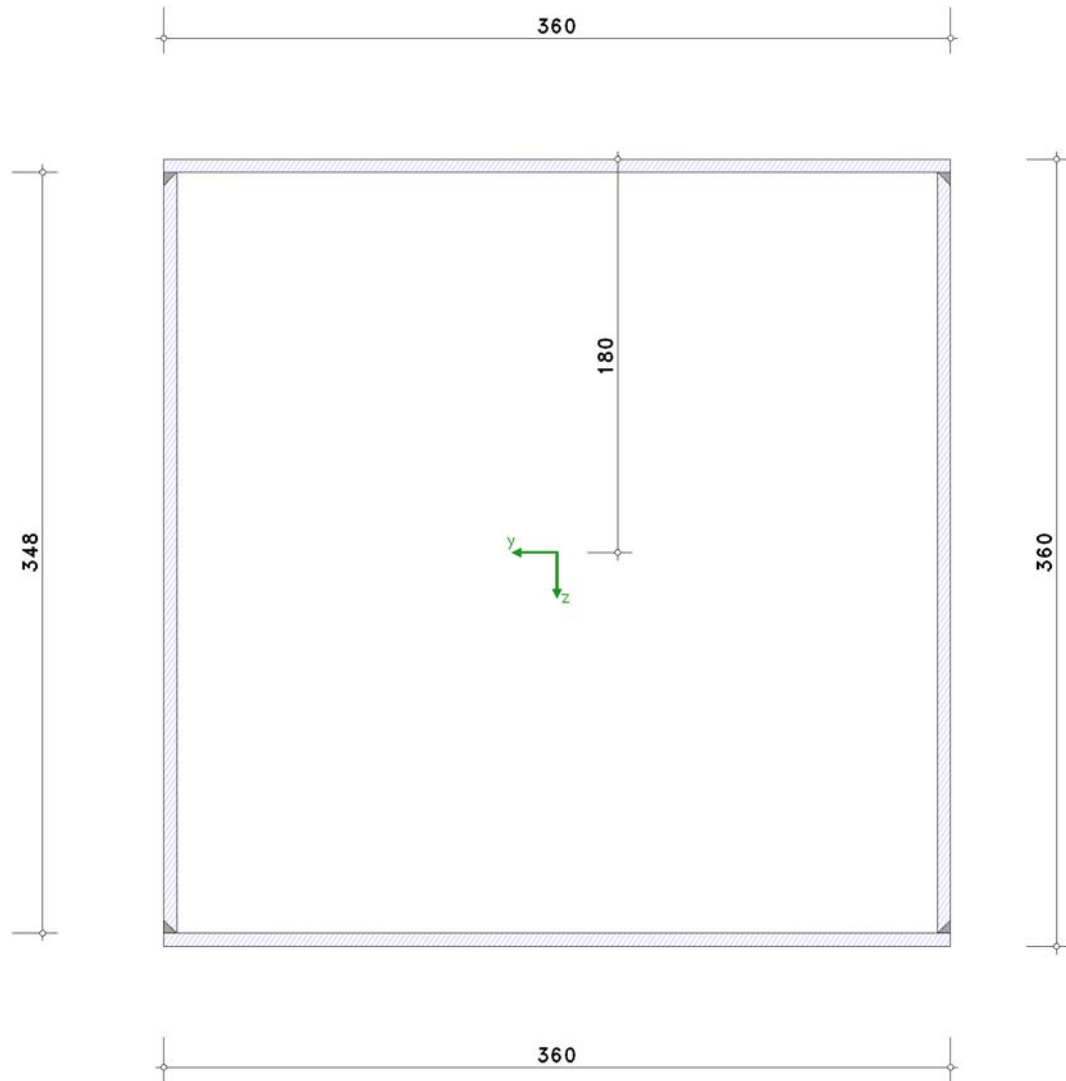
resistance of members in stability failure $\gamma_{M1} = 1.10$



verifications of buckling resistance

assumption: flange induced web buckling is excluded.

assumption: plate area is supported rigidly.



Lk 1:

method of effective cross-sectional area

EC 3-convention, compressive stresses positive

shear distortions are ignored.

cross-sectional properties: $A = 84.96 \text{ cm}^2$, $z_s = 180.0 \text{ mm}$, $I_y = 17749.84 \text{ cm}^4$, $y_s = 0.0 \text{ mm}$, $I_z = 17749.84 \text{ cm}^4$

maximum/minimum stresses: $\sigma_o = 111.8 \text{ N/mm}^2$, $\sigma_u = 111.8 \text{ N/mm}^2$

section class: 4 $\Rightarrow$ verification of plate buckling required !!

plate buckling

effective cross-sectional area for $N_{Ed} = 950.0 \text{ kN}$, $M_{Ed} = 0$

web:

section class 4 for $42.00 < c/t = 58.00$

critical buckling stress $\sigma_{cr,p} = k_\sigma \cdot \sigma_E = 225.7 \text{ N/mm}^2$, $\sigma_E = 56.4 \text{ N/mm}^2$, $k_\sigma = 4.00$

buckling slenderness ratio $\lambda_p = (f_y / \sigma_{cr,p})^{1/2} = 1.020$

reduction factor $\rho = (\lambda_p - 0.055 \cdot (3 + \psi)) / \lambda_p^2 = 0.769 \leq 1$ for $\lambda_p > 0.5 + (0.085 - 0.055 \cdot \psi)^{1/2} = 0.673$, $\psi = 1.000$

effective width $b_{c,eff} = \rho \cdot b = 267.5 \text{ mm}$ ($b_{e1} = 133.8 \text{ mm}$, $b_{e2} = 133.8 \text{ mm}$)

flange induced web buckling:

$h_w/t_w = 58.00 < (k \cdot E) / (f_y \cdot (A_w/A_{fc})^{1/2}) = 683.39$ ok. with $k = 0.55$, $A_w = 20.88 \text{ cm}^2$, $A_{fc} = 10.80 \text{ cm}^2$

flange top:

section class 4 for $42.00 < c/t = 58.00$

critical buckling stress $\sigma_{cr,p} = k_\sigma \cdot \sigma_E = 225.7 \text{ N/mm}^2$, $\sigma_E = 56.4 \text{ N/mm}^2$, $k_\sigma = 4.00$

buckling slenderness ratio $\lambda_p = (f_y / \sigma_{cr,p})^{1/2} = 1.020$

reduction factor $\rho = (\lambda_p - 0.055 \cdot (3 + \psi)) / \lambda_p^2 = 0.769 \leq 1$ for $\lambda_p > 0.5 + (0.085 - 0.055 \cdot \psi)^{1/2} = 0.673$, $\psi = 1.000$

effective width $b_{c,eff} = \rho \cdot b = 267.5 \text{ mm}$ ($b_{e1} = 133.8 \text{ mm}$, $b_{e2} = 133.8 \text{ mm}$)

flange bottom:

section class 4 for $42.00 < c/t = 58.00$

critical buckling stress $\sigma_{cr,p} = k_\sigma \cdot \sigma_E = 225.7 \text{ N/mm}^2$, $\sigma_E = 56.4 \text{ N/mm}^2$, $k_\sigma = 4.00$

buckling slenderness ratio $\lambda_p = (f_y/\sigma_{cr,p})^{1/2} = 1.020$

reduction factor $\rho = (\lambda_p - 0.055 \cdot (3 + \psi)) / \lambda_p^2 = 0.769 \leq 1$ for $\lambda_p > 0.5 + (0.085 - 0.055 \cdot \psi)^{1/2} = 0.673$, $\psi = 1.000$

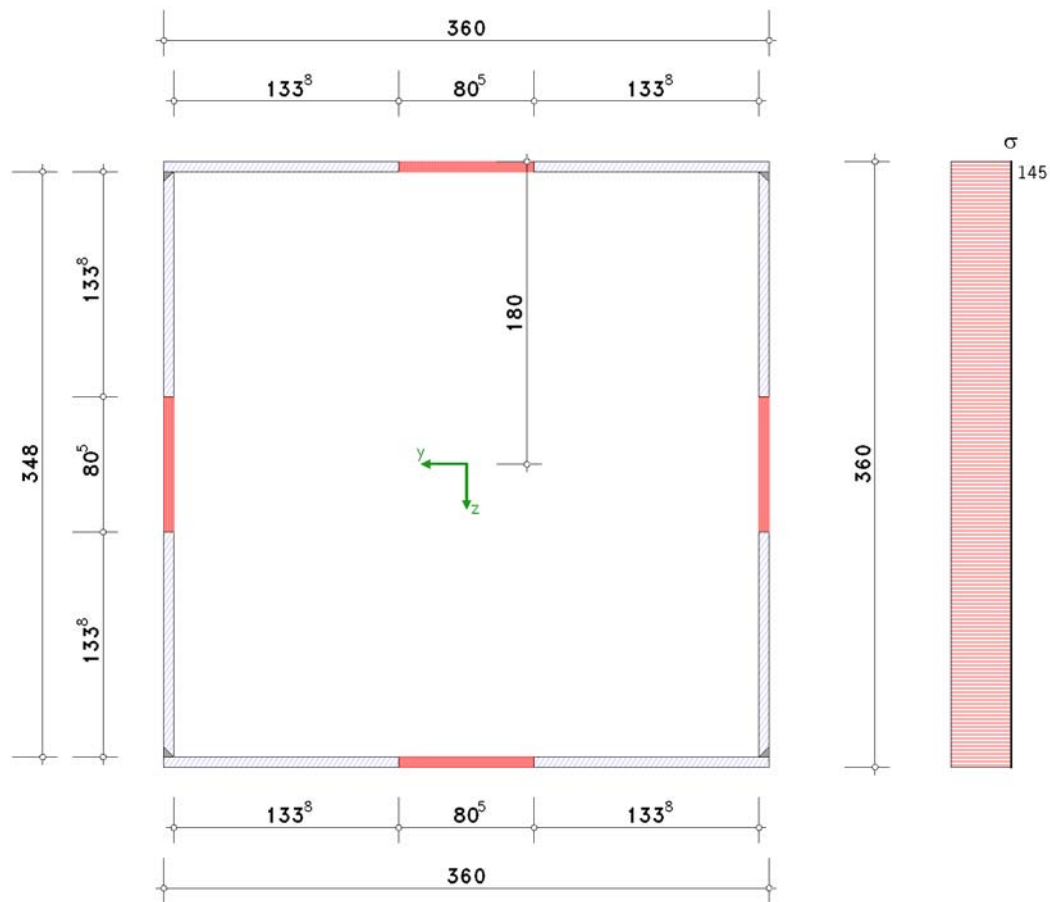
effective width $b_{c,eff} = \rho \cdot b = 267.5 \text{ mm}$ ($b_{e1} = 133.8 \text{ mm}$, $b_{e2} = 133.8 \text{ mm}$)

limit loads referring to the reduced cross-section:

distance of centroid from top $z_{s,eff} = 180.0 \text{ mm}$

cross-sectional area $A_{eff} = 65.64 \text{ cm}^2$

load capacities $N_{Rd} = (f_y \cdot A_{eff}) / \gamma_{M1} = 1402.35 \text{ kN}$



verification

$N_{Ed}/N_{Rd} = 0.677 < 1$ ok.

total utilization: $U = 0.677 < 1$ ok.

Final Result

maximum utilization:

max $U = 0.677 < 1$ ok.

assumptions:

succeeded ok.

verifications succeeded

Regulations

DIN EN 1990, Eurocode 0: Grundlagen der Tragwerksplanung;

Deutsche Fassung EN 1990:2002 + A1:2005 + A1:2005/AC:2010, Ausgabe Dezember 2010

DIN EN 1990/NA, Nationaler Anhang zur DIN EN 1990, Ausgabe Dezember 2010

DIN EN 1993-1-1, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -

Teil 1-1: Allgemeine Bemessungsregeln und Regeln für den Hochbau;

Deutsche Fassung EN 1993-1-1:2005 + AC:2009, Ausgabe Dezember 2010

DIN EN 1993-1-1/NA, Nationaler Anhang zur DIN EN 1993-1-1, Ausgabe Dezember 2010

DIN EN 1993-1-5, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -

Teil 1-5: Plattenförmige Bauteile;
Deutsche Fassung EN 1993-1-5:2006 + AC:2009, Ausgabe Dezember 2010
DIN EN 1993-1-5/NA, Nationaler Anhang zur DIN EN 1993-1-5, Ausgabe Dezember 2010