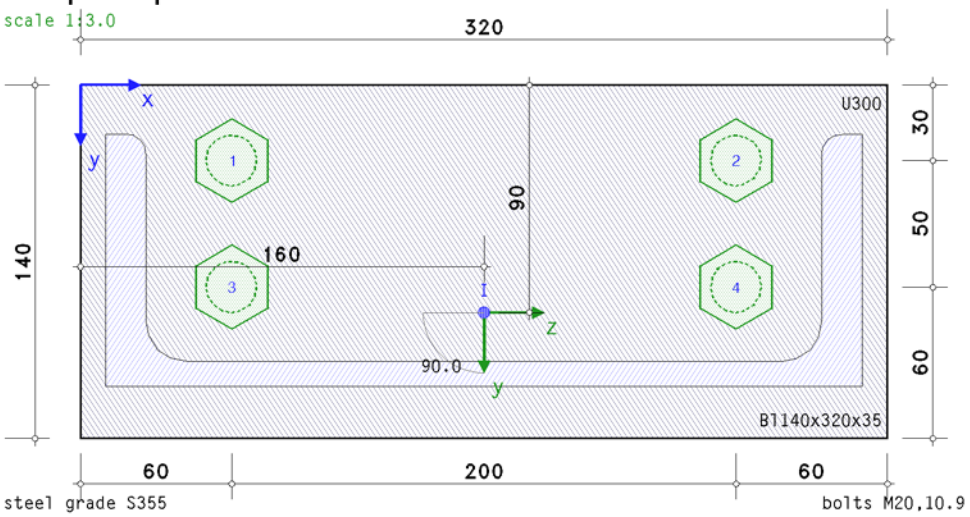
POS. 4: U

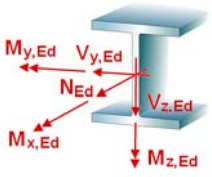
bolted end plate connection EC 3-1-8 (04.25), NA: Deutschland

1. input report



steel grade
steel grade S355
bolts
bolt class 10.9, bolt size M20, normal wrench size
thread included in the shear plane
connection
end plate (rectangular): thickness t_p = 35.0 mm, width b_p = 320.0 mm, length l_p = 140.0 mm
beam: section U300
rotation of cross-section arround section centroid β = 90.0°
beam-end plate: surrounding fillet weld, weld thickness a = 6.0 mm
coordinates of centroid of beam section at x_p = 160.0 mm, y_p = 90.0 mm
coordinates of the beam centroid on end plate x_s = 160.0 mm, y_s = 90.0 mm
bolts:
uniform arrangement of bolts, 2 vertical and 2 horizontal rows
edge distances top e_o = 30.0 mm, below e_u = 60.0 mm
distances between bolts $p_{y,1-2}$ = 50.0 mm
edge distances left e_l = 60.0 mm, right e_r = 60.0 mm
distances between bolts $p_{x,1-2}$ = 200.0 mm
calculation
verification:
calculation of internal forces and moments (FEM) and verifications of resistance
verification of end plate with the plastic method, verification of compression by contact
verification of beam section with the plastic method
verification of welds with the directional method
verification of bolts, check of distances
FEM-calculation:
bolts are plasticly calculated, spring constant of bolts c_f = 5792.5 kN/cm
plastic limit force $F_{t,f} = f_{t,f} \cdot F_{t,Rd}$ = 167.4 kN, $f_{t,f}$ = 0.950, $F_{t,Rd} = (k_2 \cdot f_{ub} \cdot A_s) / \gamma_{M2}$ = 176.26 kN, k_2 = 0.90
effective elongation at failure $\varepsilon_{t,f} = f_{t,\varepsilon} \cdot \varepsilon_{ub}$ = 2.3%, $f_{t,\varepsilon}$ = 0.250, ε_{ub} = 9.0%
without preloading ($F_{p,C}$ = 0)
effective foundation modulus of end plate c_b = 6000.0 kN/cm³
number / dimension of finite elements each direction n_x / Δx = 98 / 3.3 mm, n_y / Δy = 43 / 3.3 mm
max. 50 iteration steps (tolerance limit 5‰)
internal forces and moments refering to local axes of cross-section

Lc	N_{Ed} kN	$M_{y,Ed}$ kNm	$V_{z,Ed}$ kN	$M_{z,Ed}$ kNm	$V_{y,Ed}$ kN	$M_{x,Ed}$ kNm
1	-33.47	81.25	-21.16	-6.13	-4.35	-0.92
2	0.27	4.66	1.34	-5.73	2.40	-0.00
3	-33.24	80.85	-21.38	-5.83	-4.16	-0.89
4	0.11	4.94	1.50	-5.93	2.27	-0.02
5	-7.75	7.83	-5.09	14.00	5.80	-0.07
6	-28.26	69.16	-19.97	-7.69	-5.39	-0.93
7	-3.37	13.12	0.51	-4.63	3.13	0.00
8	-4.11	-0.63	-4.25	12.91	5.07	-0.07



partial safety factors for material
resistance of cross-sections γ_{M0} = 1.00

resistance of bolts, welds, plates in bearing γ_{M2} = 1.25

local stresses especially of the beam and of welds are not considered !

utilizations

Lc	U_p	U_σ	U_b	U_{pl}	$U_{pl,s}$	$U_{wt,s}$	$U_{t,s}$	$U_{vt,s}$	$U_{b,s}$	U_q	$U_{c/t}$	U_w	U
1	0.779	0.779	0.718	0.745	0.989	0.962	0.206	0.711	0.026	0.713	0.353	0.949	0.989*
2	0.168	0.114	0.168	0.163	0.152	0.054	0.061	0.206	0.002	0.262	0.200	0.320	0.320
3	0.760	0.760	0.707	0.742	0.980	0.933	0.206	0.710	0.025	0.698	0.349	0.931	0.980
4	0.174	0.118	0.174	0.169	0.158	0.057	0.063	0.215	0.002	0.273	0.204	0.332	0.332
5	0.276	0.276	0.143	0.250	0.271	0.082	0.092	0.322	0.006	0.619	0.281	0.750	0.750
6	0.573	0.573	0.535	0.635	0.866	0.661	0.206	0.695	0.024	0.712	0.348	0.931	0.931
7	0.116	0.111	0.116	0.124	0.182	0.073	0.081	0.274	0.002	0.263	0.206	0.333	0.333
8	0.248	0.248	0.111	0.233	0.251	0.065	0.073	0.251	0.005	0.538	0.274	0.643	0.643

U_p: utilization of end plate; U_σ: utilization of end plate due to stress; U_b: utilization of end plate due to compression by contact
U_{pl}: min. plastic utilization of the connection; $U_{pl,s}$: plastic utilization of tensile forces of bolts; $U_{wt,s}$: utilization of bolts due to elongation
$U_{t,s}$: utilization of bolts due to tension; $U_{vt,s}$: utilization of bolts due to shear; $U_{b,s}$: utilization of bolts due to bearing resistance
U_q: stress utilization of the beam; U_{ct}: c/t-utilization of the beam; U_w: utilization of welds
U: total utilization
*) maximum utilization

2. final result

maximum utilization of end plate due to 8 Lc: max U_p with corresponding values

node	x mm	y mm	u_z mm	b_z N/mm²	m_{xx} kNm/m	m_{yy} kNm/m	m_{xy} kNm/m	q_x kN/m	q_y kN/m	U_p
300	19.6	113.9	0.010	19.20	29.50	27.46	21.49	-3608.67	2935.48	0.779

x,y: node coordinates; u_z: deformations (lifting off positive); b_z: compression by contact (compression positive); m_{xx},m_{yy},m_{xy}: moments
q_x,q_y: shear forces; q_x,q_y: shear forces; U_p: utilization of end plate

maximum utilization of bolts due to 8 Lc: max U_s with corresponding values

	x mm	y mm	F_t kN	U_{wt}	U_{vt}	U_b	U_s
1	60.0	30.0	59.20	0.065	0.251	0.002	0.251
2	260.0	30.0	167.10	0.356	0.708	0.008	0.708
3	60.0	80.0	105.76	0.116	0.541	0.023	0.541
4	260.0	80.0	167.44	0.962	0.711	0.006	0.962

x,y: bolt coordinates; F_t: bolt force; U_{wt}: utilization due to elongation; U_{vt}: utilization due to shear
U_b: utilization due to bearing resistance; U_s: utilization of bolts

maximum utilization of end plate [Lc 1]	max U_p = 0.779 < 1 ok
maximum utilization of bolts due to elongation [Lc 1]	max $U_{wt,s}$ = 0.962 < 1 ok
maximum utilization of bolts [Lc 1]	max U_s = 0.989 < 1 ok
maximum utilization of the beam [Lc 1]	max (U_q,U_{ct}) = 0.713 < 1 ok
maximum utilization of welds [Lc 1]	max U_w = 0.949 < 1 ok
maximum utilization [Lc 1]	**max U = 0.989 < 1 ok**

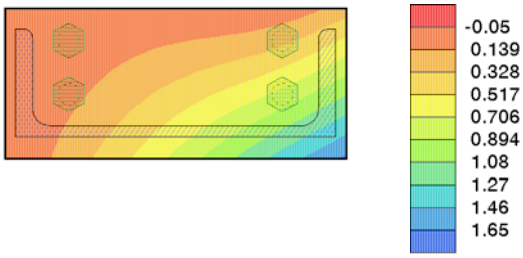
verification succeeded

3. Lc 1 (decisive)

3.1. end plate

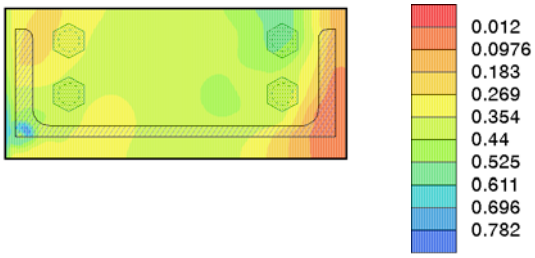
design values: N = -33.47 kN, M_y = 81.25 kNm, M_z = -6.13 kNm

deformations u_z [mm]
min u_z = -0.0535 mm, max u_z = 1.6871 mm



deformations lifting off positive

utilization of end plate U_p
min U_p = 0.012, max U_p = 0.779



utilization of end plate

node	x mm	y mm	u_z mm	U_p
300	19.6	113.9	0.010	0.779
4356	320.0	140.0	1.687	0.026

x,y: node coordinates; u_z: deformations (lifting off positive); U_p: utilization of end plate

utilization of bolts

	x mm	y mm	w_t mm	F_t kN	ε_{wt} %	U_{wt}
1	60.0	30.0	0.018	21.16	0.052	0.023
2	260.0	30.0	0.280	167.10	0.801	0.356
3	60.0	80.0	0.091	105.76	0.261	0.116
4	260.0	80.0	0.758	167.44	2.165	0.962

x,y: bolt coordinates; w_t: deformation (tension positive); F_t: bolt force; ε_{wt}: elongation
U_{wt}: utilization due to elongation

utilization of end plate [node 300] $U_{max} = 0.779 < 1$ ok
utilization of bolts due to elongation [bolt 4] $U_{s,max} = 0.962 < 1$ ok
minimum plastic utilization of the connection $U_{pl,min} = 0.745 < 1$ ok
plastic utilization of tensile forces of bolts $U_{pl,t,s} = 0.989 < 1$ ok

3.2. bolts

design values: min F_t = 21.16 kN, max F_t = 167.44 kN, V_z = -21.16 kN, V_y = -4.35 kN,
M_x = -0.92 kNm

verification of bolts
U_{tp} utilization due to punching shear failure, U_{vt} utilization due to shear in tension, U_b utilization due to bearing resistance, U utilization of bolts

bolt 1	$U_{tp,1} = 0.026$	$U_{vt,1} = 0.201$	$U_{b,1} = 0.026$	$U_1 = 0.201$
bolt 2	$U_{tp,2} = 0.205$	$U_{vt,2} = 0.708$	$U_{b,2} = 0.008$	$U_2 = 0.708$
bolt 3	$U_{tp,3} = 0.130$	$U_{vt,3} = 0.541$	$U_{b,3} = 0.023$	$U_3 = 0.541$
bolt 4	$U_{tp,4} = 0.206$	$U_{vt,4} = 0.711$	$U_{b,4} = 0.006$	$U_4 = 0.711$
total:	$U_{tp} = 0.206$	$U_{vt} = 0.711$	$U_b = 0.026$	$U = 0.711 < 1$ ok

in utilization of bolts U_{max} ist minimum plastic utilization of the connection $U_{pl,min}$ = 0.745
and plastic utilization of tensile forces of bolts $U_{pl,s}$ = 0.989 is included.

utilization of bolts $U_{max} = 0.989 < 1$ ok

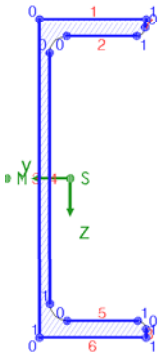
3.3. beam

calculation acc. to EC 3-1-1, 8.2 for dieses section not möglich !!
cross-section cannot be verified using plastic methods !!
elastic verification for N = -33.47 kN, M_y = 81.25 kNm, V_z = -21.16 kN, M_z = -6.13 kNm
V_y = -4.35 kN, T_t = -0.92 kNm
verification: $\sigma_v = 253.12$ N/mm² $< \sigma_{v,Rd} = 355.00$ N/mm² $\Rightarrow U_\sigma = 0.713 < 1$ ok
c/t-verification: utilization $U_{c/t} = 0.353 < 1$ ok

utilization of the beam $\max(U_\sigma, U_{c/t}) = 0.713 < 1$ ok

3.4. welds

design values: N = -33.47 kN, M_y = 81.25 kNm, V_z = -21.16 kN, M_z = -6.13 kNm,
V_y = -4.35 kN, M_x = -0.92 kNm



weld 1:	a_w = 6.0 mm	l_w = 100.0 mm
weld 2:	a_w = 6.0 mm	l_w = 66.0 mm
weld 3:	a_w = 6.0 mm	l_w = 300.0 mm
weld 4:	a_w = 6.0 mm	l_w = 236.0 mm
weld 5:	a_w = 6.0 mm	l_w = 66.0 mm
weld 6:	a_w = 6.0 mm	l_w = 100.0 mm
weld 7:	a_w = 6.0 mm	l_w = 8.0 mm
weld 8:	a_w = 6.0 mm	l_w = 8.0 mm

Max: $\sigma_{1,w,Ed} = 413.48$ N/mm² $< f_{1w,d} = 435.56$ N/mm²,
$\sigma_{2,w,Ed} = 206.73$ N/mm² $< f_{2w,d} = 352.80$ N/mm² $\Rightarrow U_w = 0.949 < 1$ ok

utilization of welds $U_{max} = 0.949 < 1$ ok

3.5. total

utilization Lc 1 $U_{max} = 0.989 < 1$ ok

4. Regulations

EN 1990, Eurocode 0: Grundlagen der Tragwerksplanung;
Deutsche Fassung EN 1990:2002 + A1:2005 + A1:2005/AC:2010, Ausgabe Dezember 2010
EN 1990/NA, Nationaler Anhang zur EN 1990, Ausgabe Dezember 2010

EN 1993-1-1, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -
Teil 1-1: Allgemeine Bemessungsregeln und Regeln für den Hochbau;
Deutsche Fassung EN 1993-1-1:2022, Ausgabe April 2025
EN 1993-1-1/NA, Nationaler Anhang zur EN 1993-1-1, Ausgabe April 2026

EN 1993-1-8, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -
Teil 1-8: Bemessung von Anschlüssen; Deutsche Fassung EN 1993-1-8:2024, Ausgabe April 2025
EN 1993-1-8/NA, Nationaler Anhang zur EN 1993-1-8, Ausgabe April 2026

prEN 1993-1-14, Eurocode 3: Bemessung und Konstruktion von Stahlbauten -
Teil 1-14: Bemessung mithilfe von Finite-Element-Berechnung;
Deutsche und englische Fassung prEN 1993-1-14:2023, Ausgabe September 2023