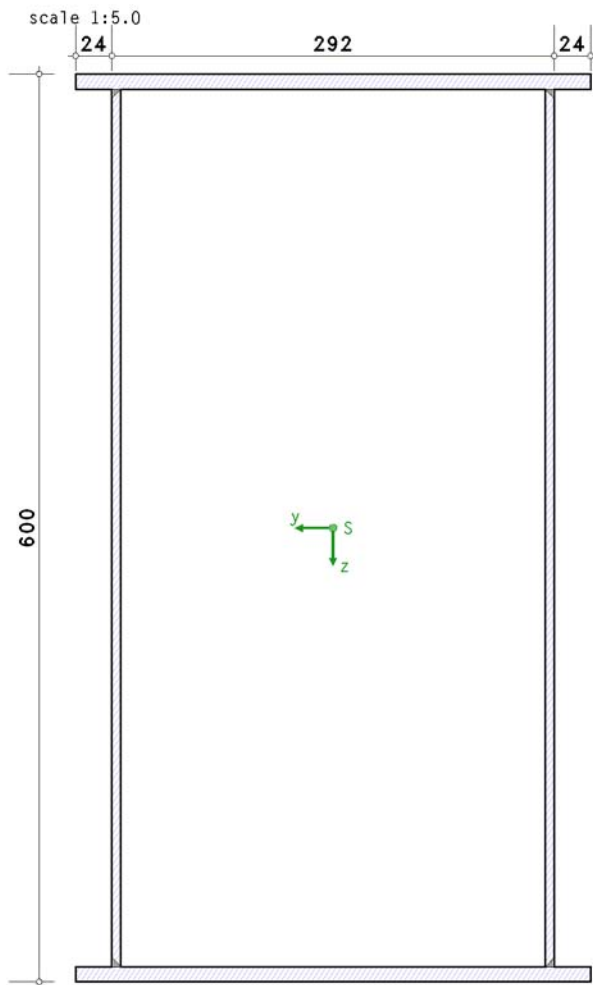


POS. 17: KINDMANN/KRÜGER 2.8.5

verification of cross-section EC 3-1-8 (12.10), NA: Deutschland

1. input report



steel

steel grade S355

material safety factor

resistance of cross-sections $\gamma_{M0} = 1.00$

geometry

section parameters (box section):

overall depth $h = 600.0$ mm, web thickness $t_w = 6.0$ mm, flange width $b_f = 292.0$ mm

projection $b_{\bar{u}} = 24.0$ mm, flange thickness $t_f = 10.0$ mm

resistance

elastic verification, calculation as thin-walled cross-section

valid normal, shear-, equivalent stress: $\sigma_{x,Rd} = 355.0$ N/mm², $\tau_{Rd} = 205.0$ N/mm², $\sigma_{v,Rd} = 355.0$ N/mm²

internal forces and moments referring to local axes of cross-section

σ -generating forces (N, M) work at centroid, τ -generating forces (V, T_t) work at shear center

Lk 1: $M_{y,Ed} = 560.00$ kNm, $V_{z,Ed} = 140.00$ kN

notes

buckling is not investigated.

2. Lk 1

2.1. verification of cross-section

internal forces and moments: $M_y = 560.00$ kNm, $V_z = 140.00$ kN

2.1.1. elastic verification

elastic verification for $M_y = 560.00 \text{ kNm}$, $V_z = 140.00 \text{ kN}$

elastic stresses: $\max |\sigma_x| = 213.49 \text{ N/mm}^2$, $\max \tau = 22.32 \text{ N/mm}^2$, $\max \sigma_v = 213.87 \text{ N/mm}^2$

$\max \sigma_x$ bei $y = 170.0 \text{ mm}$, $z = 300.0 \text{ mm}$: $\sigma_x = 213.49 \text{ N/mm}^2$, $\tau = 0.00 \text{ N/mm}^2$, $\sigma_v = 213.49 \text{ N/mm}^2$

$\min \sigma_x$ bei $y = 170.0 \text{ mm}$, $z = -300.0 \text{ mm}$: $\sigma_x = -213.49 \text{ N/mm}^2$, $\tau = 0.00 \text{ N/mm}^2$, $\sigma_v = 213.49 \text{ N/mm}^2$

$\max \tau$ bei $y = 143.0 \text{ mm}$, $z = -0.0 \text{ mm}$: $\sigma_x = -0.00 \text{ N/mm}^2$, $\tau = 22.32 \text{ N/mm}^2$, $\sigma_v = 38.66 \text{ N/mm}^2$

$\max \sigma_v$ bei $y = 143.0 \text{ mm}$, $z = -300.0 \text{ mm}$: $\sigma_x = -213.49 \text{ N/mm}^2$, $\tau = -7.41 \text{ N/mm}^2$, $\sigma_v = 213.87 \text{ N/mm}^2$

valid equivalent stress: $\sigma_{v,Rd} = 355.0 \text{ N/mm}^2$

verification: $\sigma_v = 213.87 \text{ N/mm}^2 < \sigma_{v,Rd} = 355.00 \text{ N/mm}^2 \Rightarrow U_\sigma = 0.602 < 1$ **ok**

c/t-ratio: outstand flange: utilization $U_{c/t} = 0.165 < 1$ **ok**

internal compression parts: utilization $U_{c/t} = 0.733 < 1$ **ok**

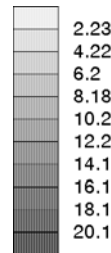
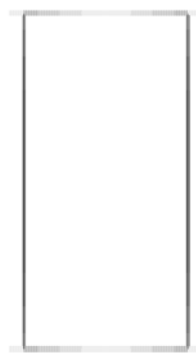
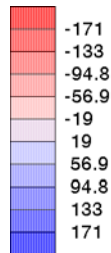
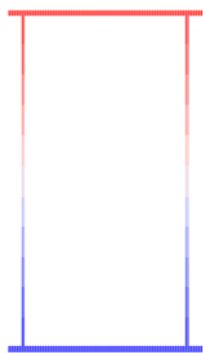
total: utilization $U_{c/t} = 0.733 < 1$ **ok**

normal stresses σ_x [N/mm²]

$\min \sigma_x = -213.4$, $\max \sigma_x = 213.4$

shear stresses τ [N/mm²]

$\max \tau = 22.3$

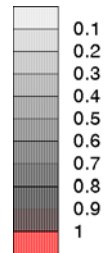
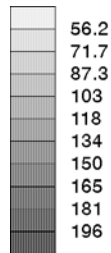


equivalent stresses σ_v [N/mm²]

$\max \sigma_v = 213.7$

utilization U_σ

$\max U_\sigma = 0.602$



stresses, utilizations

y mm	z mm	σ_x N/mm ²	τ N/mm ²	σ_v N/mm ²	U_σ
0.0	0.0	-213.36	0.00	213.36	0.601
-27.0	0.0	-213.36	7.41	213.75	0.602
-313.0	0.0	-213.36	7.41	213.75	0.602

y mm	z mm	σ_x N/mm ²	τ N/mm ²	σ_v N/mm ²	U_σ
0.0	600.0	213.36	0.00	213.36	0.601
-24.0	300.0	0.00	22.32	38.66	0.109

y,z: node coordinates; σ_x, τ, σ_v : stresses; U_σ : stress utilization

3. final result

maximum utilization:

stress

$\max U_\sigma = 0.602 < 1$ **ok**

c/t-ratio

$\max U_{c/t} = 0.733 < 1$ **ok**

resistance

$\max U = 0.733 < 1$ **ok**

verification succeeded