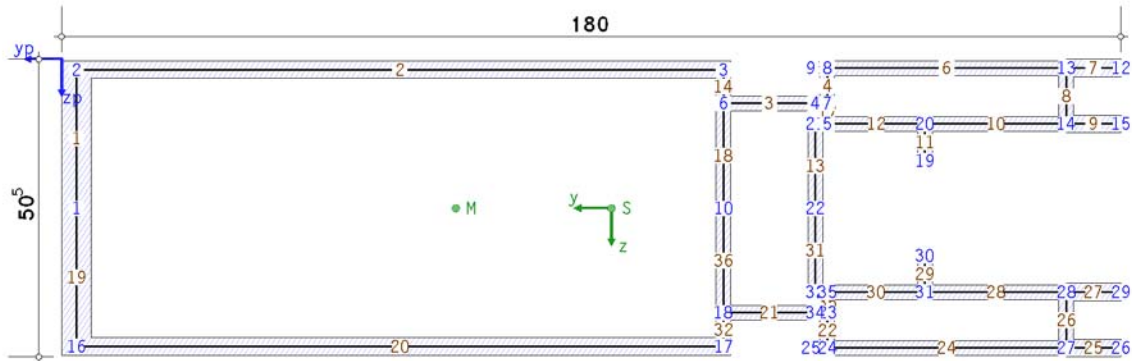


POS. 32: LAUFS/WHEELBECK A.2

verification of cross-section EC 3-1-8 (12.10), NA: Deutschland

1. input report



aluminium

alloy 6060, T6, $t \leq 15$ mm, calculation parameters:

char. value of 0,2%-yield strength $f_0 = 140.0$ N/mm²

char. tensile strength $f_u = 170.0$ N/mm²

elastic modulus $E = 70000.0$ N/mm²

material safety factor

resistance of cross-sections $\gamma_{M0} = 1.10$

geometry

dimensions of cross-section: $y_{min} = 0.0$ mm, $y_{max} = 180.0$ mm, $z_{min} = -25.3$ mm, $z_{max} = 25.3$ mm

width,height of cross-section: $b = 180.0$ mm, $h = 50.5$ mm

centroid, rotation angle of main axis: $e_y = 86.5$ mm, $e_z = 0.0$ mm, $\alpha = 0.00^\circ$



cross-sectional area	A	=	17.94	cm ²
bending moment of inertia	I_η	=	69.59	cm ⁴
bending moment of inertia	I_ζ	=	612.54	cm ⁴
section modulus	$W_{\eta+}$	=	27.56	cm ³
section modulus	$W_{\eta-}$	=	27.56	cm ³
section modulus	$W_{\zeta+}$	=	65.54	cm ³
section modulus	$W_{\zeta-}$	=	70.78	cm ³
torsional moment of inertia	I_T	=	125.40	cm ⁴

y_p, z_p : coordinates of nodes (due to 4H-QUER)

	y_p mm	z_p mm		y_p mm	z_p mm		y_p mm	z_p mm		y_p mm	z_p mm		y_p mm	z_p mm
1	-2.5	25.3	12	-180.0	1.5	22	-128.1	25.3	23	-130.1	43.0	30	-146.8	33.3
2	-2.5	1.7	13	-170.8	1.5	4	-128.1	7.5	24	-130.1	49.0	31	-146.8	39.5
3	-112.5	1.7	14	-170.8	11.0	5	-130.1	11.0	25	-127.3	49.0	32	-128.1	39.5
6	-112.5	7.5	15	-180.0	11.0	10	-112.5	25.3	26	-180.0	49.0	34	-128.1	43.0
7	-130.1	7.5	19	-146.8	17.3	16	-2.5	48.8	27	-170.8	49.0	35	-130.1	39.5
8	-130.1	1.5	20	-146.8	11.0	17	-112.5	48.8	28	-170.8	39.5			
9	-127.3	1.5	21	-128.1	11.0	18	-112.5	43.0	29	-180.0	39.5			

p_a, p_e : points numbers at the beginning, end; d: plate thickness

	p_a	p_e	d mm		p_a	p_e	d mm		p_a	p_e	d mm		p_a	p_e	d mm
1	1	2	5.00	10	14	20	2.50	19	1	16	5.00	28	28	31	2.50
2	2	3	3.00	11	19	20	2.50	20	16	17	3.00	29	30	31	2.50
3	6	4	2.50	12	20	5	2.50	21	18	34	2.50	30	31	35	2.50
4	7	8	2.50	13	21	22	2.50	22	23	24	2.50	31	32	22	2.50
5	8	9	2.50	14	3	6	2.50	23	24	25	2.50	32	17	18	2.50
6	8	13	2.50	15	4	7	2.50	24	24	27	2.50	33	34	23	2.50
7	12	13	3.00	16	5	21	2.50	25	26	27	3.00	34	35	32	2.50
8	13	14	2.50	17	5	7	2.50	26	27	28	2.50	35	35	23	2.50
9	14	15	3.00	18	6	10	2.50	27	28	29	3.00	36	18	10	2.50

resistance

elastic verification, calculation with FE-method

valid normal, shear-, equivalent stress: $\sigma_{x,Rd} = 127.3$ N/mm², $\tau_{Rd} = 73.5$ N/mm², $\sigma_{v,Rd} = 139.4$ N/mm²

internal forces and moments referring to local axes of cross-section

σ -generating forces (N, M) work at centroid, τ -generating forces (V, T_t) work at shear center

Lk 1: $N_{Ed} = -4.30$ kN, $M_{z,Ed} = 6.59$ kNm, $V_{y,Ed} = 3.77$ kN

notes

aluminium alloy cross-sections must be at least in class 3.

buckling is not investigated.

2. Lk 1

2.1. verification of cross-section

internal forces and moments: $N = -4.30$ kN, $M_z = 6.59$ kNm, $V_y = 3.77$ kN

2.1.1. elastic verification

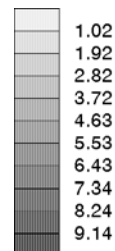
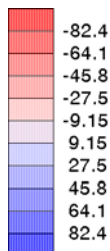
elastic verification for $N = -4.30$ kN, $M_z = 6.59$ kNm, $V_y = 3.77$ kN

normal stresses σ_x [N/mm²]

min $\sigma_x = -102.9$, max $\sigma_x = 90.7$

shear stresses τ [N/mm²]

max $\tau = 10.2$

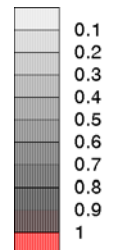
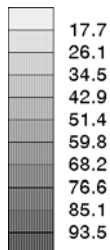


equivalent stresses σ_v [N/mm²]

max $\sigma_v = 103.0$

utilization U_σ

max $U_\sigma = 0.809$



strains, stresses, utilizations

y mm	z mm	ϵ ‰	σ_x N/mm ²	τ N/mm ²	σ_v N/mm ²	U_σ
0.0	0.2	-1.539	-102.94	0.08	102.94	0.809
0.0	25.3	-1.539	-102.94	0.00	102.94	0.809
0.0	50.3	-1.539	-102.94	0.08	102.94	0.809
-180.0	50.5	1.313	90.71	0.01	90.71	0.713
-180.0	47.5	1.313	90.71	0.01	90.71	0.713
-113.7	6.3	0.277	19.41	10.16	26.20	0.188
0.0	5.6	-1.539	-102.94	0.94	102.96	0.809

y,z: node coordinates; σ_x, τ, σ_v : stresses; U_σ : stress utilization

verifications

max $|\sigma_x| = 102.94$ N/mm² < $\sigma_{x,Rd} = 127.27$ N/mm² $\Rightarrow U_{\sigma_x} = 0.809 < 1$ ok

max $\tau = 10.16$ N/mm² < $\tau_{Rd} = 73.48$ N/mm² $\Rightarrow U_\tau = 0.138 < 1$ ok

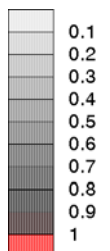
max $\sigma_v = 102.96$ N/mm² < $\sigma_{v,Rd} = 139.42$ N/mm² $\Rightarrow U_{\sigma_v} = 0.738 < 1$ ok

total: max $U_\sigma = 0.809 < 1$ ok

3. final result

maximum stress utilization max U_σ due to 1 Lk

max max $U_\sigma = 0.809$



maximum stress utilization max U_σ due to 1 Lk

y mm	z mm	ε ‰	σ _x N/mm ²	τ N/mm ²	σ _y N/mm ²	U _σ
-0.0	0.2	-1.539	-102.94	0.08	102.94	0.809

y,z: node coordinates; ε: cross-sectional strains; σ_x,τ,σ_y: stresses; U_σ: stress utilization

maximum utilization: resistance max U = 0.809 < 1 **ok**

verification succeeded