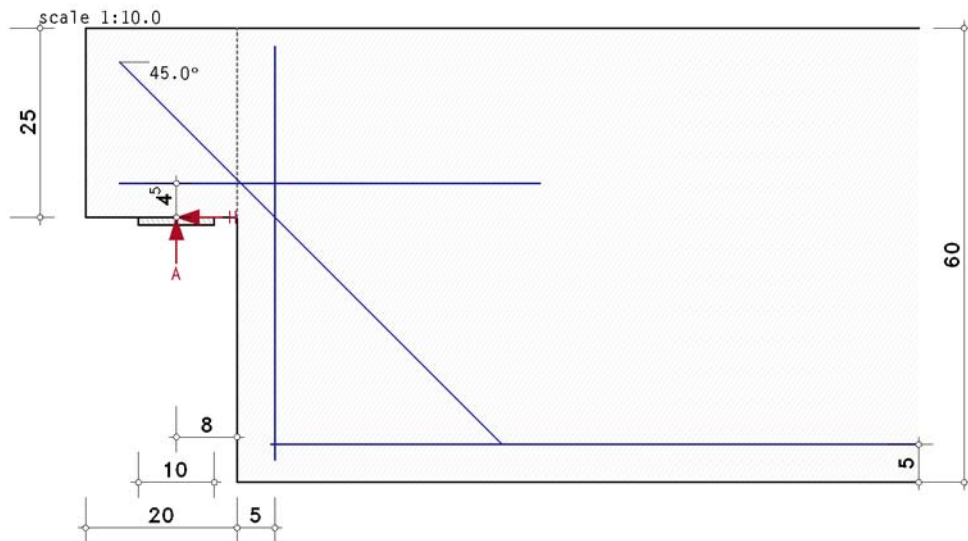


POS. 24: BEARING BRACKET M. TRANSVERSE REINFORCEMENT

dimensioning a bearing bracket EC 2 (1.11), NA: Deutschland

1. input protocol



cross section (single bracket)

beams: $b = 23.0$ cm, $h = 60.0$ cm, center distance (design calculation): $d_1 = 5.0$ cm

bracket: $l_k = 20.0$ cm, $h_k = 25.0$ cm, center distance (design calculation): $d_{1k} = 4.5$ cm

bearing plate: $l_p = 10.0$ cm, $b_p = 20.0$ cm

material properties

concrete: C40/50, reinforcement: B500A

material safety factors

design situation: basic combination

design resistance: concrete $\gamma_c = 1.50$, reinforcement $\gamma_s = 1.15$

parameters

Load application for $\Delta a = 8.0$ cm, $\Delta h = 0.0$ cm

design method acc. to F. Leonhardt

bearing bracket, load application with variant 2

allocation number 0.58 (load percentage of vertical reinforcement)

inclination angle of transverse reinforcement 45.0°

bearing contact pressure: bearing joint of a plain/elastomeric bearing

design calculation values (ULS)

lc 1: $A_{Ed} = 125.0$ kN, $H_{Ed} = 25.0$ kN

2. note

general reinforcement rules are not taken into account.

3. design calculation

3.1. lc 1

method no longer state of the art !

design calculation values: $A = A_v = 125.00$ kN ($\Delta a = 8.0$ cm), $H = A_h = 25.00$ kN ($\Delta h = 0.0$ cm)

bearing contact pressure (plain/elastomeric bearing): $\sigma_p = 6.25$ N/mm² < $\sigma_{Rd,max} = 18.13$ N/mm² **ok**

load percentage of vertical reinforcement $A_v = 0.58 \cdot A = 72.50$ kN

tensile reinforcement horizontal: $Z_{A+H} = 114.30$ kN $\Rightarrow$ req $A_{s,h} = 2.63$ cm²

vertical: $Z_v = 60.90$ kN $\Rightarrow$ req $A_{s,v} = 1.40$ cm²

dimensions of beam nose:

58% vertical suspension: min $h_k = 5.6$ cm < h_k **ok**

42% transverse suspension: min $d_k = 4.0$ cm < d_k **ok**

verification of compression strut: min $b = 7.8$ cm < b **ok**

tensile splitting reinforcement: $A_{sb,h}$ and $A_{sb,v}$ constructive

load percentage of transverse reinforcement $A_s = 52.50$ kN at $\alpha = 45.0^\circ$

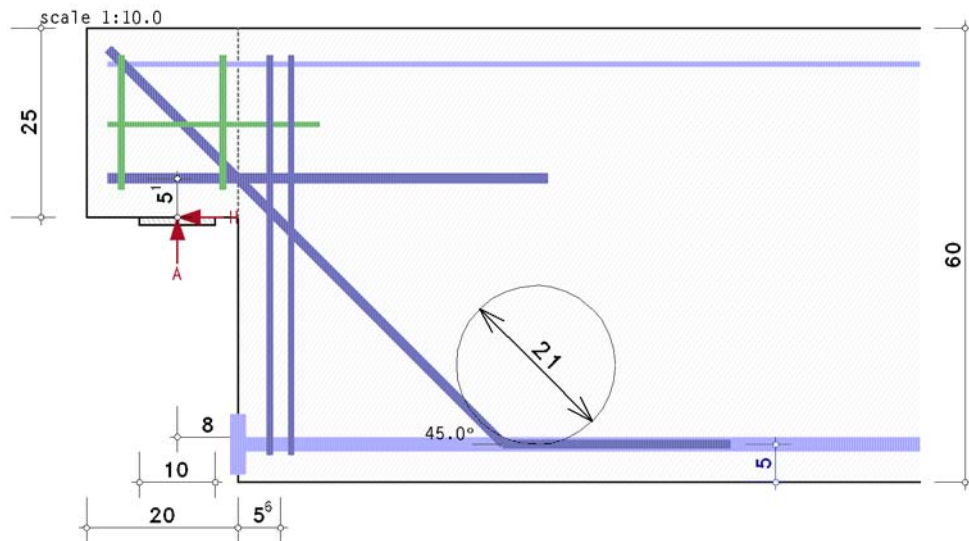
tensile reinforcement transverse: $Z_s = 74.25$ kN $\Rightarrow$ req $A_{s,s} = 1.71$ cm²

total: $A_{s,h} = 2.63$ cm², $A_{s,v} = 1.40$ cm², $A_{s,s} = 1.71$ cm² ($\Sigma A_{st} = 5.74$ cm²)

4. final result

maximum reinforcement: $A_{s,h} = 2.63 \text{ cm}^2$, $A_{s,v} = 1.40 \text{ cm}^2$, $A_{s,s} = 1.71 \text{ cm}^2$

5. selected reinforcement



calculation of the required anchorage lengths: bonding conditions good

concrete cover $c_{v,v} = 3.6 \text{ cm}$

concrete cover $c_{v,h} = 2.8 \text{ cm}$

bottom reinforcement above rod reinforcement, $2\text{Ø}8$, exst $A_{s0} = 1.01 \text{ cm}^2$

bottom reinforcement bottom rod reinforcement, $3\text{Ø}20$, exst $A_{su} = 9.42 \text{ cm}^2$

anchorage length: req $l_v = 20.0 \text{ cm} > \text{exst } l_v = 4.6 \text{ cm}$ **not ok !!**

main reinforcement $1\text{Ø}14$, $D_{\min} = 5.6 \text{ cm}$, exst $A_{s,h} = 3.08 \text{ cm}^2 > \text{req } A_{s,h} = 2.63 \text{ cm}^2$ **ok**

(U-bents, 2-shear) anchorage lengths: bracket req $l_v = 16.5 \text{ cm} > \text{exst } l_v = 14.2 \text{ cm}$ **not ok !!**

beams $l_v = 35.3 \text{ cm}$

transverse reinforcement $1\text{Ø}12$, $D_{\min,hor} = 4.8 \text{ cm}$, $D_{\min,ver} = 21.0 \text{ cm}$, exst $A_{s,s} = 2.26 \text{ cm}^2 > \text{req } A_{s,s} = 1.71 \text{ cm}^2$ **ok**

(U-bents, 2-shear) anchorage lengths: bracket req $l_v = 18.7 \text{ cm} < \text{exst } l_v = 20.1 \text{ cm}$ **ok**

beams $l_v = 30.0 \text{ cm}$

tensile splitting reinforcement horizontal $1\text{Ø}8$ (U-bents, 2-shear), exst $A_{sb,h} = 1.01 \text{ cm}^2$

anchorage length: $l_v = 8.0 \text{ cm} \Rightarrow \text{length of U-bents: } l_s = 29.0 \text{ cm}$

tensile splitting reinforcement vertical $2\text{Ø}8$ (stirrup, 2-shear), exst $A_{sb,v} = 2.01 \text{ cm}^2$

vertical reinforcement $2\text{Ø}8$ (stirrup, 2-shear), exst $A_{s,v} = 2.01 \text{ cm}^2 > \text{req } A_{s,v} = 1.40 \text{ cm}^2$ **ok**

anchoring horiz. $2\text{Ø}10$ (U-bents, 2-shear), exst $A_{s,h1} = 3.14 \text{ cm}^2$

anchoring vertical $2\text{Ø}8$ (stirrup, 2-shear), exst $A_{sb,v1} = 2.01 \text{ cm}^2$

center distance exst $d_{1k} = 5.10 \text{ cm} > \text{clc } d_{1k} = 4.5 \text{ cm}$ **!!**

center distance exst $e_1 = 5.60 \text{ cm} > \text{clc } e_1 = 5.0 \text{ cm}$ **!!**

center distance exst $d_{1s} = 5.00 \text{ cm}$ (anchoring of transverse reinforcement in the beam)

check center distance, calculation model does not correspond to the design !

design resistance not guaranteed !!

s. selected reinforcement

6. regulations

EN 1990, Eurocode 0: Grundlagen der Tragwerksplanung;

Deutsche Fassung EN 1990:2002 + A1:2005 + A1:2005/AC:2010, Ausgabe Dezember 2010

EN 1990/NA, Nationaler Anhang zur EN 1990, Ausgabe Dezember 2010

EN 1992-1-1, Eurocode 2: Bemessung und Konstruktion von Stahlbeton- und Spannbetonbauteilen -

Teil 1-1: Allgemeine Bemessungsregeln und Regeln für den Hochbau;

Deutsche Fassung EN 1992-1-1:2004 + AC:2010, Ausgabe Januar 2011

EN 1992-1-1/NA, Nationaler Anhang zur EN 1992-1-1, Ausgabe April 2013

Fritz Leonhardt, Eduard Mönig: Vorlesungen über Massivbau, Zweiter Teil,

Sonderfälle der Bemessung im Stahlbetonbau, Dritte Auflage, Springer-Verlag, 1986

Fritz Leonhardt, Eduard Mönig: Vorlesungen über Massivbau, Dritter Teil,

Grundlagen zum Bewehren im Stahlbetonbau, Dritte Auflage, Springer-Verlag, 1977