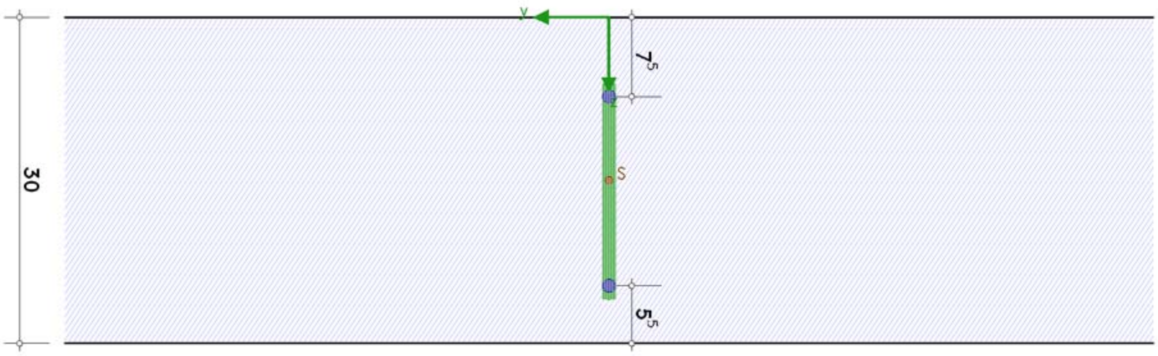
POS. 40: BSP. 8.5 P (LOHMEYER)

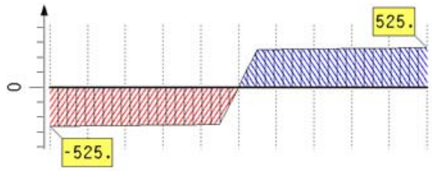
reinforced concrete design EC 2 (1.11), NA: Deutschland 4H-EC2QB Version: 10/2023-1b

1. input protocol

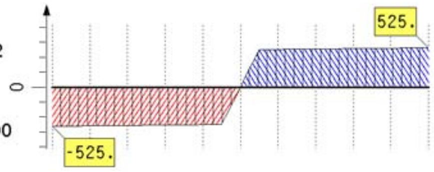


1.1. building material

reinforcing steel bottom B500A, above B500A, concrete C30/37
characteristic values for the consideration of creep and shrinkage in concrete (for verifications in SLS):
creep coefficient φ_{eff} = 2.367, shrinkage $\varepsilon_{cs,\infty}$ = 0.000‰
characteristic values for the calculation of effective concrete strengths (crack verification): cement CEM 32.5 R (class N), effective section thickness h_0 = 30.0 cm
stress-strain line of reinforcing steel: EC 2-1-1, 3.2.7 (bilinear)

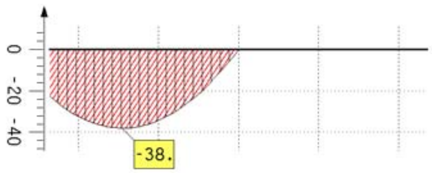


bottom:
σ in N/mm²
ε in ‰
max ε = 25.00
max σ = 525.00



above:
σ in N/mm²
ε in ‰
max ε = 25.00
max σ = 525.00

stress-strain line of concrete: EC 2-1-1, 3.1.5 (realistic)



σ in N/mm²
ε in ‰
min ε = -11.79
min σ = -38.00

1.2. material safety factors

serviceability: concrete γ_c = 1.00, reinforcement γ_s = 1.00

1.3. cross section

plate: h = 30.0 cm
axis distances: d_o = 7.3 cm, d_u = 5.3 cm
base reinforcement: A_{so0} = Ø 12 / 90.0 = 1.26 cm²/m, A_{su0} = Ø 12 / 90.0 = 1.26 cm²/m, a_{sbv0} = Ø 12 / 100.0 = 1.13 cm²/m (1-cut)
max. reinforcement ratio ρ_s = 8.00%

1.4. durability and concrete cover

above: minimum strength class, concrete cover for $Ø_s$ = 12 mm, $Ø_{sb}$ = 12 mm
due to reinforcement corrosion XD3 ⇒ C35/45, c_{min} = 40 mm, Δc = 15 mm, $c_{nom} = c_{min}+\Delta c$ = 55 mm
due to concrete attack XM1 ⇒ C12/15
minimum concrete quality C35/45 with f_{ck} = 35.0 N/mm² > 30.0 N/mm² not ok !!
minimum axial spacing min d = $c_{nom}+Ø_{sb}+Ø_s/2$ = 73 mm ≤ clc d = 73 mm ok
bottom: minimum strength class, concrete cover for $Ø_s$ = 12 mm, $Ø_{sb}$ = 12 mm
due to reinforcement corrosion XC3 ⇒ C20/25, c_{min} = 20 mm, Δc = 15 mm, $c_{nom} = c_{min}+\Delta c$ = 35 mm
minimum concrete quality C20/25 with f_{ck} = 20.0 N/mm² < 30.0 N/mm² ok
minimum axial spacing min d = $c_{nom}+Ø_{sb}+Ø_s/2$ = 53 mm ≤ clc d = 53 mm ok

1.5. design parameters

1.5.1. crack verification

perm. crack width: $w_{o,lim}$ = 0.30 mm, $w_{u,lim}$ = 0.30 mm
bar diameter of the crack-distributing reinforcement: $Ø_{ro}$ = 12 mm, $Ø_{ru}$ = 12 mm

1.5.1.1. minimum reinforcement (EC 2, 7.3.2)

calculation acc. to Lohmeyer/Ebeling
time of crack initiation $t_{crit} = 1.2 \cdot t_{max,T} + 20 = 49$ h,
$t_{max,T}$ = 24 h for normal hardening concrete (CEM 32.5 R) and h_0 = 30.0 cm
concrete tensile strength during initial cracking $f_{ct,eff} = k_{ct} \cdot f_{ctm}$ = 1.89 N/mm²,
coefficient $k_{ct} = k_j \cdot k_{ct(t)}$ = 0.65 for normal hardening concrete (CEM 32.5 R) and t_{crit} = 49 h, $k_{ct(t)}$ = 0.65,
k_j = 1.0 (spring/autumn)
calculation of the constraint force for base plates
dimensions of the base plate L_p = 25.00 m, B_p = 18.00 m, t_p = 30.0 cm
separation cracking in base plates (friction model)
constraint force $N_{ct,1} = \gamma_{ct} \cdot \mu_d \cdot \sigma_0 \cdot L_p/2$ = 520.00 kN/m, γ_{ct} = 1.00
design value of friction $\mu_d = \gamma_R \cdot \mu_0$ = 0.100, γ_R = 1.25, μ_0 = 0.08
design value of ground compression $\sigma_{0d} = t_p \cdot \rho_p + q_p$ = 416.00 kN/m², ρ_p = 0.0 kN/m³, q_p = 416.00 kN/m²
constraint force N_{ct} = 520.00 kN/m
constraint force (EC 2): $N_{ct,EC2} = f_{ct,eff} \cdot A_{c,eff}$ = 541.91 kN/m, $f_{ct,eff}$ = 1.89 N/mm², $A_{c,eff}$ = 2860.0 cm²/m
constraint force $N_{ct} \le N_{ct,EC2}$ $\Rightarrow$ crack resistance available ($N_{ct}/N_{ct,EC2}$ = 0.960 < 1)
calculation of minimum reinforcement with $N_{ct,clc} = N_{ct} \cdot A_c / A_{c,eff}$ = 545.45 kN/m, A_c = 3000.0 cm²/m, $A_{c,eff}$ = 2860.0 cm²/m
coefficient for stress distribution k_c from centric constraint
coefficient for consideration of nonlinearly distributed residual stresses k from self-induced constraint

2. notes

crack verification: perm. crack width (load+constraint) is not verified.
cross section type plate: results refer to 1 m plate width.

3. crack verification

material properties
σ-ε line acc. to EC 2, 3.1.5(1): C30/37, ε_{c1} = -2.16‰, ε_{cu1} = -3.50‰, f_{cm} = 38.00 N/mm², E_{cm} = 32836.6 N/mm²
σ-ε line acc. to EC 2, 3.2.7(2a): B500A, ε_u = 25.00‰, f_{yk} = 500.0 N/mm², f_{tk} = 525.0 N/mm², E_s = 200000.0 N/mm²
σ-ε line acc. to EC 2, 3.2.7(2a): B500A, ε_u = 25.00‰, f_{yk} = 500.0 N/mm², f_{tk} = 525.0 N/mm², E_s = 200000.0 N/mm²

3.1. calculation of minimum reinforcement (EC 2, 7.3.2)

crack stress σ_{cr} = 1.82 N/mm², coefficient for non-linear residual stresses k = 0.80
reinforcement above
perm. crack width $w_{o,lim}$ = 0.30 mm
coefficient for stress distribution k_{co} = 1.00, tension zone A_{cto} = 15.00 dm², crack zone $A_{c,eff,o}$ = 15.00 dm²
stress in reinforcement σ_{sro} = 233.5 N/mm²
reinforcement (minimum reinforcement) $A_{so,min}$ = 9.34 cm²
reinforcement bottom
perm. crack width $w_{u,lim}$ = 0.30 mm
coefficient for stress distribution k_{cu} = 1.00, tension zone A_{ctu} = 15.00 dm², crack zone $A_{c,eff,u}$ = 13.60 dm²
stress in reinforcement σ_{sru} = 233.5 N/mm²
reinforcement (minimum reinforcement) $A_{su,min}$ = 9.34 cm²

4. final result

maximum reinforcement: A_{so} = 9.34 cm², A_{su} = 9.34 cm²

design resistance not guaranteed !!
s. durability and concrete cover

5. regulations

EN 1990, Eurocode 0: Grundlagen der Tragwerksplanung;
Deutsche Fassung EN 1990:2002 + A1:2005 + A1:2005/AC:2010, Ausgabe Dezember 2010
EN 1990/NA, Nationaler Anhang zur EN 1990, Ausgabe Dezember 2010

EN 1992-1-1, Eurocode 2: Bemessung und Konstruktion von Stahlbeton- und Spannbetonbauteilen -
Teil 1-1: Allgemeine Bemessungsregeln und Regeln for den Hochbau;
Deutsche Fassung EN 1992-1-1:2004 + AC:2010, Ausgabe Januar 2011
EN 1992-1-1/NA, Nationaler Anhang zur EN 1992-1-1, Ausgabe April 2013

G. Lohmeyer, K. Ebeling: Weiße Wannen - einfach und sicher, Planung und Konstruktion
wasserundurchlässiger Bauwerke aus Beton, Verlag Bau+Technik GmbH, Düsseldorf